WALNUT RIDGE

REZONING APPLICATION

Located in:

Montgomery County, Virginia

Project Number: 3246.0

Date: March 31, 2023



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REZONING APPLICATION



Rezoning Application Form Rezoning, Conditional Zoning, Proffer Amendment Montgomery County, Virginia 755 Roanoke St. Suite 2A, Christiansburg, VA 24073 540-394-2148 | mcplan@montgomerycountyva.gov

Count I, Vindinin			
Application Request: (Please check one) ☐ Condition	U	coning	
Applicant Information: (PLEASE PRINT – if addition		lditional sheets)	
Owner of Record (attach separate page for add'l owners): SHAH Development, LLC	Address: P.O. Box 1499, Christiansburg, VA 24068		
Telephone: (540) 381-8429	Email: kconner@shahllc.com		
	Y		
Applicant Name: Owner Contract Purchaser/Lessee SHAH Development, LLC	Address: P,O. Box 1499, Christiansburg, VA 24068		
Telephone: (540) 381-8429	Email: kconner@shahllc.com		
D	T A LL		
Representative Name and Company: Foresight Design Services	Address: 1260 Radford Street, Christiansburg, VA 24073		
Telephone: (540) 381-6011	Email: info@foresightdesignservices.com		
Property Description:			
Location or Address: (Describe in relation to nearest inters	,		
Property lies east of the intersection of Houchins Road and	Crosscreek Drive		
Parcel ID Number(s): 018437, 018441	Acreage: 26.718	Existing Zoning: M1	
Comprehensive Plan Designation:	Existing Use:		
Urban Expansion Area	Vacant Lot/ Wooded area		
Description of Request: (Please provide additional information Proposed Zoning (Include Acreage):	ion on attached sheet if necessary	"	
RM1			
Proposed Use:			
Residential - Townhomes			
I certify that the information supplied on this application is accurate and true to the best of my knowledge. In accemployees of Montgomery County and State of Virginiand reviewing the above application. If signing on behalf of a Corporation, Partnership, or LLC, produced the companion of the companion of the corporation of the corpo	ddition, I hereby grant perm ia to enter the above proper please specify your title with	ission to the agents and ty for the purposes of processing	
/// monnaging Men	bec	3-29-23	
Owner 1/Signature		Date	
Owner 2 Signature (for add'l owners please attach sepa	rate sheet)	Date	
(M) managing Memb	ex 2	3-29-13	
Applicant Signature		Date	
, John T. Neel		3/31/23	
Representative/Agent Signature	Date		

REZONING APPLICATION JUSTIFICATION

Additional Rezoning Requirements

The applicant for rezoning shall provide a statement of justification to address the following items in the application materials to demonstrate what impact the proposed request will have on the County's resources and how the request complies with Montgomery County's Comprehensive Plan.

Section 10-54(1)(k)(4), Montgomery County Zoning Ordinance

- 4. Zoning Map Amendments. If the application is for a reclassification of property to a different zoning district classification on the Zoning Map, the applicant shall address all the following in its statement of justification or plat unless not applicable. The Planning Commission shall give reasonable consideration to the following matters:
 - a. Whether the proposed zoning district classification is consistent with the Comprehensive Plan.
 - Response: RM-1 is consistent with the Urban Expansion designation for the Comprehensive Plan. Urban Expansion per the Comprehensive Plan is for the development of residential and non-residential projects.
 - b. Whether there are any changed or changing conditions in the area affected that make the proposed rezoning appropriate.
 - Response: With Phase III of Walnut Creek approved and ready to build, the current site will compliment Walnut Creek and provide further expansion with additional residential housing in this area.
 - c. Whether the range of uses in the proposed zoning district classification are compatible with the uses permitted on other property in the immediate vicinity.
 - Response: The RM-1 zoning for residential is similar to the adjacent Walnut Creek Phase III residential townhomes.
 - d. Whether adequate utility, sewer and water, transportation, school and other facilities exist or can be provided to serve the uses that would be permitted on the property if it were rezoned.
 - Response: All utility, water, sewer, transportation and schools are available for the proposed rezoned property.
 - e. The effect of the proposed rezoning on the County's ground water supply.
 - Response: There are no effects to ground water sources.
 - f. The effect of uses allowed by the proposed rezoning on the structural capacity of the soils.
 - Response: There is no expected impact on the structural capacity of the soils on site.

- g. The impact that the uses that would be permitted if the property were rezoned will have upon the volume of vehicular and pedestrian traffic and traffic safety in the vicinity and whether the proposed rezoning uses sufficient measures to mitigate the impact of through construction traffic on existing neighborhoods and school areas.

 Response: A full traffic impact analysis has been performed and is in the appendices of this rezoning. The TIA has been previously submitted to VDOT.
- h. Whether a reasonably viable economic use of the subject property exists under the current zoning.

Response: The current zoning does provide for industrial use of the existing site; however, to match the adjacent growth of the area and the increase in housing along Houchins road, the residential aspect of the proposed rezoning compliments the Urban Expansion as well as ties into the existing nearby subdivisions. The existing residential traffic on Houchins Road as well as the topography of the subject property does not economically lend itself to an industrial use.

- i. The effect of the proposed rezoning on environmentally sensitive land or natural features, wildlife habitat, vegetation, water quality and air quality.
 - Response: The proposed subdivision will replace the existing wooded area and will have construction activity that will create noise, air pollutants from construction equipment, and removed trees and vegetation from the area. However, once the subdivision is complete, stormwater bioretention facilities will preserve the water quality, street trees and buffer trees will be planted to restore some of the possible habitat lost as well as help with air quality.
- j. Whether the proposed rezoning encourages economic development activities in areas designated by the Comprehensive Plan and provides desirable employment and enlarges the tax base.
 - Response: The proposed rezoning will provide residential housing per the Urban Expansion area designated in the Comprehensive Plan. This will help to increase the population and taxbase.
- k. Whether the proposed rezoning considers the needs of agriculture, industry, and businesses in future growth.
 - Response: The proposed rezoning seeks to match the trend of residential housing in the expansion area that will provide additional growth for businesses in the area.
- I. Whether the proposed rezoning considers the current and future requirements of the community as to land for various purposes as determined by population and economic studies.
 - Response: Per the Comprehensive Plan, the urban expansion need for the area is residential and nonresidential. The zoning for this area is typically either industrial or residential along the Town of Christiansburg corporate line. Therefore, by providing residential housing the current and future requirements of the urban expansion area are met.

m. Whether the proposed rezoning encourages the conservation of properties and their values and the encouragement of the most appropriate use of land throughout the County.

Response: The proposed rezoning tends to focus on the appropriate use for expansion and growth in this area of Montgomery County per the Comprehensive Plan. Providing additional housing within the urban expansion area will serve to lessen the pressure for development in areas of the County that are more rural in nature and lack services such as water and sewer.

n. Whether the proposed rezoning considers trends of growth or changes, employment, and economic factors, the need for housing, probable future economic and population growth of the county.

Response: This rezoning considers growth of the area and the needs for residential housing to help with population growth in Montgomery County.

o. The effect of the proposed rezoning on the provision of moderate housing by enhancing opportunities for all qualified residents of Montgomery County.

Response: The proposed townhome development will provide housing opportunities for residents of Montgomery County and in particular the added housing capacity serves to provide movement among all different economic categories through 2nd or 3rd order impacts.

p. The effect of the rezoning on natural, scenic, archaeological, or historic features of significant importance.

Response: No known features will be impacted.

Comprehensive Plan Justification

Introduction:

Any development within Montgomery County is viewed by the Board of Supervisors, Planning Commission, County Staff, and Citizens through the prism of the comprehensive plan. The following narrative and analysis will address points within the comprehensive plan and discuss how the proposed use aligns with the vision, goals, and objectives of the comprehensive plan. Please note that below are excerpts from the adopted 2025 Comprehensive Plan and one should refer to the Plan for the full text.

The subject property is identified in the Comprehensive Plan as part of the Urban Expansion Area. The text below is from the Comprehensive Plan, with the bold text demonstrating how the proposal meets the guidelines. Additional comprehensive references are included within the narrative.

Overview:

The project proposes to allow for 26.718 acres of land be developed into residential housing located north of the Town of Christiansburg Industrial Park near the Walnut Creek Subdivision area in Montgomery County. The entirety of the 26.718 acres is vacant and wooded terrain which is located along Houchins Road. The western area is bounded by Houchins Road and Industrial areas of the Town as well as the Walnut Creek Subdivision. To the north is residential properties, the east by woods, and the south is the corporate limits for the Town as well as industrial properties. The subdivision will consist of up to 199 townhomes with recreational open space amenities, bus access, sidewalks, private roadways as well as utilities infrastructure and stormwater management facilities. Stormwater management will be achieved through the use of an onsite detention facility. Stormwater quality will be handled by the dedication of open space on the site, bio retention cells, and nutrient credits.

Policy Chapters:

Planning and Land Use

PLU 1.8.3 Urban Expansion Area Land Use:

- a. Urban Expansion Areas are the preferred location for new residential and nonresidential development occurring in the unincorporated areas of Montgomery County.
 - Discussion This proposed rezoning will meet the preferred location for residential development.

PLU 1.8.5 Urban Expansion Area Facilities and Utilities:

- a. Urban Expansion Areas are or will be served by public sewer and water services provided by the County or by the towns and the city, by mutual agreement.
 - Discussion The subdivision will be connected to both water and sewer services through coordination efforts with the Town and Christiansburg and the Montgomery County PSA.

Conclusion:

The Walnut Ridge Subdivision fits within the Comprehensive Plan for Montgomery County's Urban Expansion Areas by providing residential housing with utilities from both the County and Town.

Narrative

Section 1: Project Background

Rezoning Request:

The property owner, SHAH Development, requests to rezone property identified as Tax Parcel Numbers 080-A-44 and 080-A-46 to Multiple Family- Residential (RM-1) for the development of Walnut Ridge. Currently, the property is zoned Industrial Manufacturing (M-1).

The rezoning request will propose up to 199 townhomes that will consist of three-bedroom units. In addition to residential housing, the project includes construction of new private roads, sidewalks, utility infrastructure, stormwater management facilities, and recreational amenities.

The subject property is identified in the Comprehensive Plan as a future land use of Urban Expansion as well as the property is located adjacent to the corporate limits of the Town of Christiansburg. The property rezoning would support the Comprehensive Plan for expansion and provide residential housing to the area.

Zoning Details:

- Area: Approximately 26.718 acres total (RM-1)
- Address: Houchins Road, Christiansburg, VA
- Parcel ID: 018437 and 018441
- Tax Parcel Numbers: 080-A-44 and 080-A-46
- Magisterial District: Shawsville
- Current Zoning: M-1
- Maximum Density (per zoning): 8.00 Dwellings/Acres (Townhomes)
- Future Land Use: Urban Expansion Area
- Flood Zone: Entire property is located outside of the FEMA 100-year flood zone
- Open Space: +/- 4.00 acres (15% of property, per zoning)

Property History:

In 2021, the property was purchased by SHAH development, which was owned by the Sawyers Family who own and operate Sawyers Bus Sales.

Existing Property Conditions:

As can be seen in the following photographs, the existing property is vacant wooded land though zoned M-1, it has not been developed. The majority of the site is wooded and is in close proximity to the Town of Christiansburg corporate limits and nearby residential developments, such as Walnut Creek. The property contains rolling topography that can easily accommodate residential development.

The Town of Christiansburg corporate limits makes up the southern property boundary. Undeveloped agricultural lands adjoin the property to the east. Residential homes and farmland is located to the north. Houchins Road along with industrial and residential development form the western property boundary.

PHOTOS



Proposed Entrance Facing South on Houchins.



Proposed Entrance Facing North on Houchins.



Internal wooded areas of site.



Internal timber areas previously cut.



Stream near northeast corner of property.



View of entrance to Walnut Creek Subdivision from northwest end of property.

Section 2: Walnut Ridge RM-1 Narrative

Rezoning Narrative

Walnut Ridge consists of residential development that includes single family attached (townhomes), as well as roads, sidewalks, trails, recreation areas, and open space to serve the residents of the development.

The requested RM-1 rezoning will keep to the Comprehensive Plan for Urban Expansion and help Montgomery County achieve its long-term goals related to land development and housing. This RM-1 rezoning includes a Concept Plan that depicts the location, quantity, and layout of the various elements to be included in Walnut Ridge.

Voluntary proffers are included in this rezoning request to mitigate any impacts that are directly related to the development. The proffers are included in the appendices.

Single-family attached dwellings (townhomes) are designed to appeal to buyers who want a smaller home, smaller lot, and desire to have minimal exterior maintenance responsibilities such as yard upkeep, roof repair, etc. This is also appealing to homeowners who want to be close to the Town limits for access to commercial businesses, a variety of restaurants and major roadways for commutes.

In addition to creating a quality neighborhood, the RM-1 zoning district allows The Walnut Ridge development to be designed and built to achieve the goals identified in various County plans, as is discussed in a later section of this narrative. The proposed development provides numerous benefits to the County, including greater tax revenue, additional housing options for citizens, and residential development near the eastern end of Christiansburg.

Transportation

Walnut Ridge is in close proximity to Christiansburg, U.S. Route I-81, and U.S. Route 460, which makes commuting to Virginia Tech (the area's largest employer) and other parts of Montgomery County very easy. This location is ideal for many families in the New River Valley that commute for work, events, and shopping.

A Traffic Impact Study was completed for the property in 2023 and is included as part of this rezoning in the appendicies.

The development will provide an adequate bus stop for the new subdivision, coordination with VDOT and Montgomery County Public Schools. This bus stop will be located along Houchins Road and shown on the Conceptual plan.

A location for cluster mailboxes is shown on the Conceptual Plan to facilitate mail delivery and provide adequate room for access for residents. Additional parking was added to this area to allow for vehicle access to the mailboxes.

Utilities

Public water and sewer are available to serve the property and there is capacity and connections available to serve the development. Each residential lot will be connected to the public water and sewer system. The water and sewer systems will be designed and built to Town of Christiansburg and Montgomery County PSA Sewer standards. The property owner will be responsible for all costs associated with the design and construction of the water and sewer infrastructure.

An availability letter from the PSA is included in the appendices of this rezoning application. The proposed development is within the Town of Christiansburg Water Service Agreement Area.

Emergency Services

Walnut Ridge property is approximately 3.6 miles (6-10 minutes) from Christiansburg Fire Department. Fire hydrants will be installed throughout the development, at locations approved by Montgomery County, to ensure adequate fire protection is available for residents. There are no anticipated negative impacts to emergency services by developing the property. Two entrances to the development are provided as well as interconnectivity of streets providing multiple routes to access any residence within the community.

Schools

Walnut Ridge residents would attend Montgomery County Public schools.

Walnut Ridge will work closely with Montgomery County Public Schools to coordinate the location for a school bus stop. A proposed stop with shelter is shown on the master plan.

Section 3: Compliance with Adopted County Plans

Comprehensive Plan:

The property is designated as Urban Expansion on the Future Land Use Map, which allows for a variety of commercial and residential uses. Walnut Ridge will contain townhomes which is in keeping with the Future Land Use designation for the property.

Walnut Ridge is consistent with the Montgomery County Comprehensive Plan and will help the County move towards achieving goals identified in the Plan.

The Comprehensive Plan identifies future population projections for the County and the need for new housing units in the unincorporated portions of the County. Urban Expansion Areas are the preferred location for new residential and nonresidential development to occur near town boundaries. These areas are set up to accommodate a full range of residential types which include Townhomes.

Walnut Ridge focuses growth in close proximity to the Town of Christiansburg, which helps control growth outside of areas that are easily served by public services. The property is also

near U.S. Route I-81 and 460. The development can easily be served by water, sewer, and public road access, which limits the costs to the County to accommodate projected growth.

Walnut Ridge meets the goals of the Urban Expansion Area and policies identified in the Comprehensive Plan for Urban Expansion Areas (PLU 1.8). Each one of these items is directly addressed in Walnut Ridge Rezoning, as follows:

- Creates high quality residential design to be compatible with existing neighborhoods.
- Provides a residential expansion to the area that was previously approved by the Board of Supervisors as the Walnut Creek Subdivision.
- Will be served by public water and public sewer.

Walnut Ridge addresses the goals identified in the Housing Section of the Comprehensive Plan (HSG 1.1 and 1.3). One of the primary issues identified in the Comprehensive Plan with regards to housing is the "provision of safe and livable neighborhoods and communities." Walnut Ridge creates a compact, livable, pedestrian oriented housing development with open space and sidewalks to encourage residents to be outside and connected to the goings on of the community.

Walnut Ridge meets the goals of the Transportation Section of the Comprehensive Plan (TRN 1.3 and 1.4). The internal roadway network does not include any cul-de-sacs, has interconnected streets and contains pedestrian oriented facilities. The new roads connect at two points along a VDOT roadway, providing for safe and efficient flow of traffic.

Walnut Ridge rezoning is designed to meet the goals and objectives identified in the Comprehensive Plan.

Section 4: Walnut Ridge Concept Plan Details

Concept Plan

Walnut Ridge Concept Plan is included as an exhibit in the appendices. Specific elements of the Concept Plan are further described in this section. Once approved by the Montgomery County Board of Supervisors, Walnut Ridge will be developed in general conformance with the Concept Plan and this narrative.

Land Uses and Land Development Standards

Walnut Ridge will include Townhome lots that will conform to the RM-1 Zoning District Standards.

Dwelling Units and Density

The maximum total number of dwelling units for the entire RM-1 shall be 199. The 26.718-acre property will have up to an overall maximum density of 7.45 units per acre. This is lower than

the maximum density of 8.0 units per acre allowed in the Montgomery County Zoning Ordinance for RM-1.

Adjacent Properties

The Concept Plan has been designed to minimize negative impacts to the adjacent property owners and to allow the development to blend into the existing development patterns. This rezoning request and Concept Plan minimizes potential negative impacts to adjacent property owners, as the property owner wants to create a high-quality development that can be integrated into the fabric of the community.

Trash Collection

Each townhome will have trash cans for private weekly roadside collection.

Parking

Overall, there will be a minimum of 3 parking spaces per townhome.

Internal Transportation Network

Walnut Ridge shall be served by all private roads once they are completed. The minor roads in the RM-1 district shall be private and privately maintained. Each lot shall have direct road access, as is depicted on the Concept Plan.

There will be two roadway connections along Houchins Road.

Open Space

Open space for the project will include sidewalk, landscaped areas, stormwater management facilities, bio retention facilities, active open space, and other undeveloped property.

As is depicted on the Concept Plan, the main focus of open space for this development is the large central green space central to the townhomes. Approximately 6 acres of open space for the common area is set aside in the north and east corner of the site. This area encircles the stormwater management facilities and provides areas for a pedestrian trail through the area. There is also one (1) 10,000 sqft active recreational area for active recreational use adjacent to the biorentention areas. This will provide the community with a playground area for children, grills for cookouts and other amenities such as the potential for a basketball court or other active recreational amenity. The trail will connect the active recreational area with the rest of the open space and the sidewalks throughout the townhome subdivision. The sidewalk system within the development also provides direct access to another central green space of about 2.5 acres.

Buffers will be provided to meet the zoning requirements along the property lines of the subdivision. A type 4 side and rear buffer will be utilized for the areas along the corporate town limits that are adjacent to more industrial properties in the south. A type 2 side and rear buffer will be utilized for properties in the A-1 agricultural areas to the east and north of the site.

Stormwater Management

The development includes a robust stormwater management system to adequately capture, treat, and convey stormwater runoff on the property. The system will consist of a combination of a single large detention pond, several bioretention cells to maintain water quality, nutrient credit purchases, and the remaining areas to be permanent open space.

In order to protect the downstream site conditions, the detention pond and bioretention basin cells will capture the increased volume of stormwater and reduce the peak runoff to the streams. This will allow the stormwater to be discharged in a manner that does not drastically change the natural water characteristics or cause erosion. Reduction of storm events will be used to maintain acceptable flow volumes to minimize soil erosion. Pollutants will be removed from the stormwater runoff through various bioretention measures and by utilizing the open space to ensure a high level of water quality standards are met. After construction, a combination of BMP quality measures, such as bioretention areas, will be used to reduce phosphorous loading. Nutrient credits will be purchased; however, no more than 25% of the total nutrient load reduction requirements would be purchased.

On-site stormwater management will meet all applicable Montgomery County regulations, as well as Virginia Department of Environmental Quality regulations. This system will also be maintained by the Home Owners Association after construction is completed.

APPENDICES

APPENDIX A: VOLUNTARY PROFFERS

Walnut Ridge PROFFER STATEMENT

March 31, 2023

Proffer Statement for the Rezoning Application for Tax Parcels 080-A 44 and 080-A-46 (the "Property") from Industrial Manufacturing (M-1) to Multiple-Family Residential (RM-1).

Pursuant to Section 10-54(1) of the Montgomery County Zoning Ordinance, the Owner hereby voluntarily proffers that the Property which is the subject of this Rezoning Application will be developed in accordance with the following conditions, if and only if, approval is granted, and the property is rezoned as requested. This Proffer Statement shall replace in its entirety any Proffer Statement previously approved for the Property. The Applicant, the Owners, their Successors and Assigns, voluntary proffer the following conditions for the Property as follows:

1. Conceptual Layout

The Property shall be developed in general conformance with the Concept Plan prepared by Foresight Design Services, dated March 31, 2023 (the "Concept Plan").

Density

A maximum density of no more than 7.45 units per acre shall be permitted for the entire RM-1 for Townhomes.

3. Maximum Dwelling Units

A maximum of 199 single family attached (townhouses) shall be constructed.

4. Utilities

The Property shall be served by Town of Christiansburg Public Water and Montgomery County Public Service Authority sanitary sewer.

5. Property Management

A property management company and/or homeowner's association shall maintain all community owned grounds, including but not limited to landscaped areas, recreational areas, parking and paved areas, sidewalks, and stormwater management areas.

6. Bus Shelter

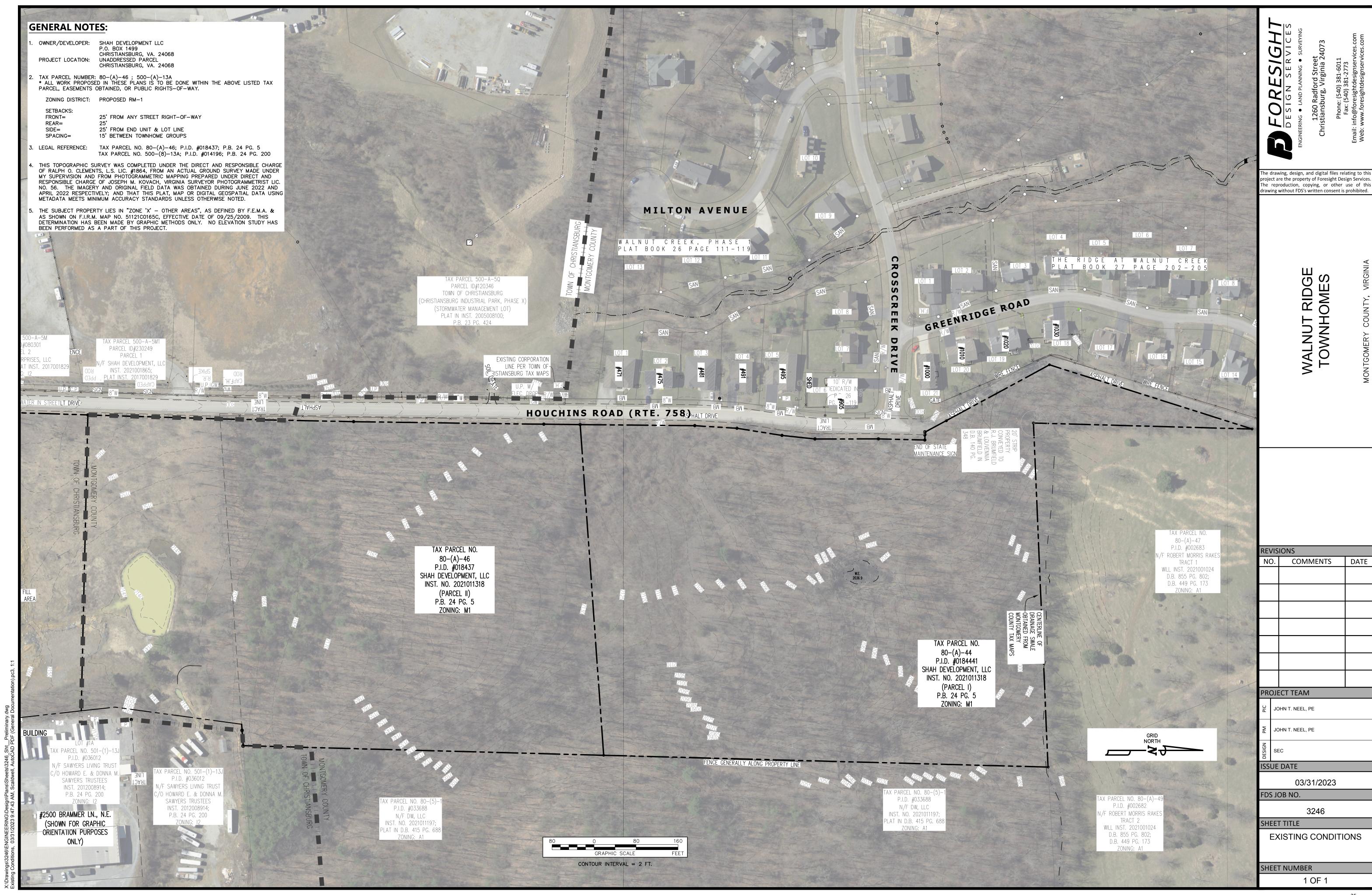
Bus shelter shall be a minimum of 5' X 10' in size, constructed of durable, architecturally sound materials that will withstand continual exposure to the elements along Houchins Road. Specific locations, style, and size to be determined in conjunction with the final site plan and coordinated with Montgomery County Public School staff. Coordination with VDOT will be required to provide suitable bus access to the shelter.

-Signature Page to Follow-

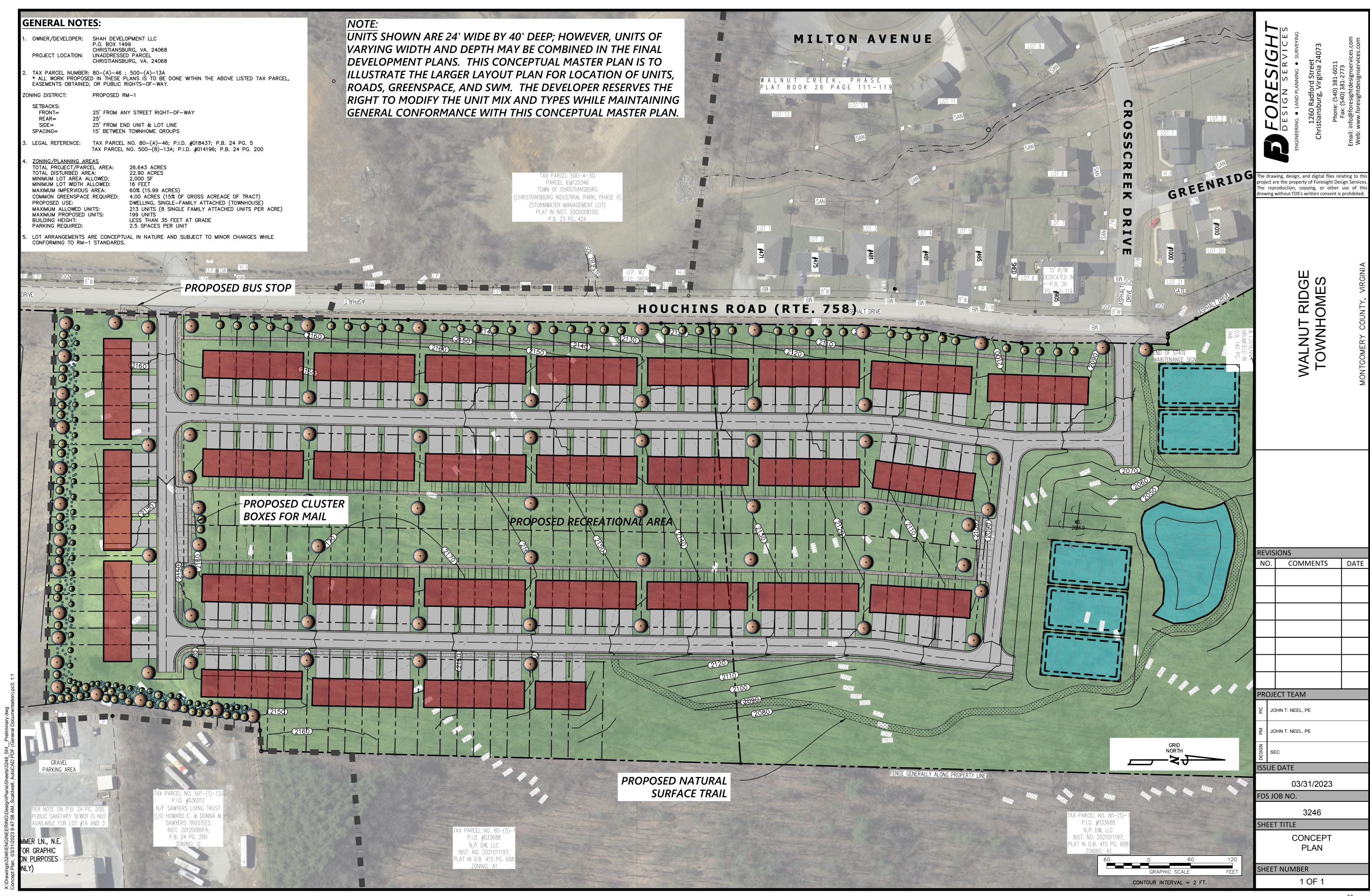
accordance with the conditions set forth in this submission.
SHAH Development, LLC
By: Name: David Hagan its:
Commonwealth of Virginia County of Montgomery
The foregoing instrument was acknowledged before me this day of
Notary Public (Seal)
My commission expires

I (we) hereby proffer that the development of the subject property of this application shall be in strict

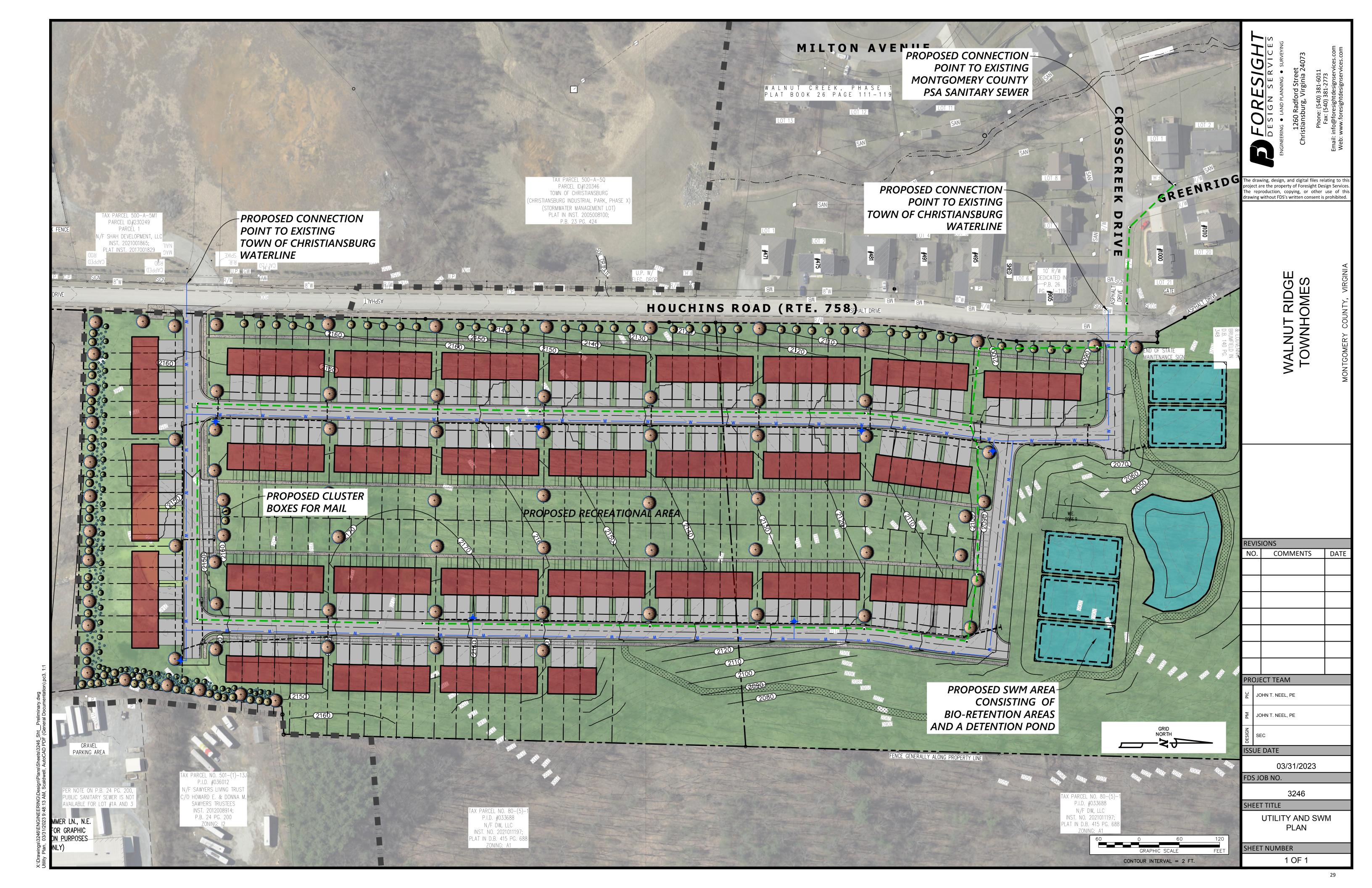
APPENDIX B: EXISTING CONDITIONS



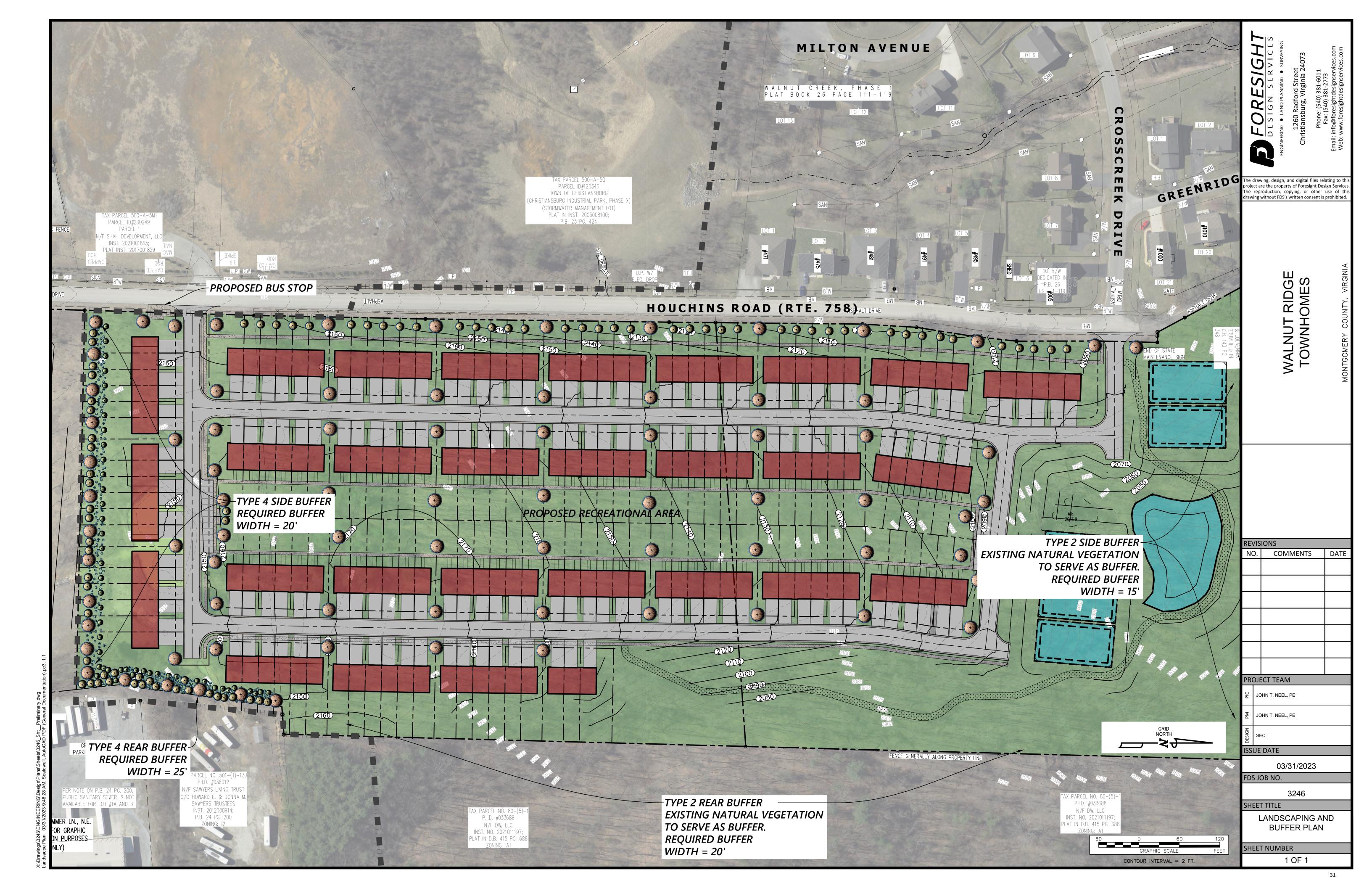
APPENDIX C: CONCEPT PLAN

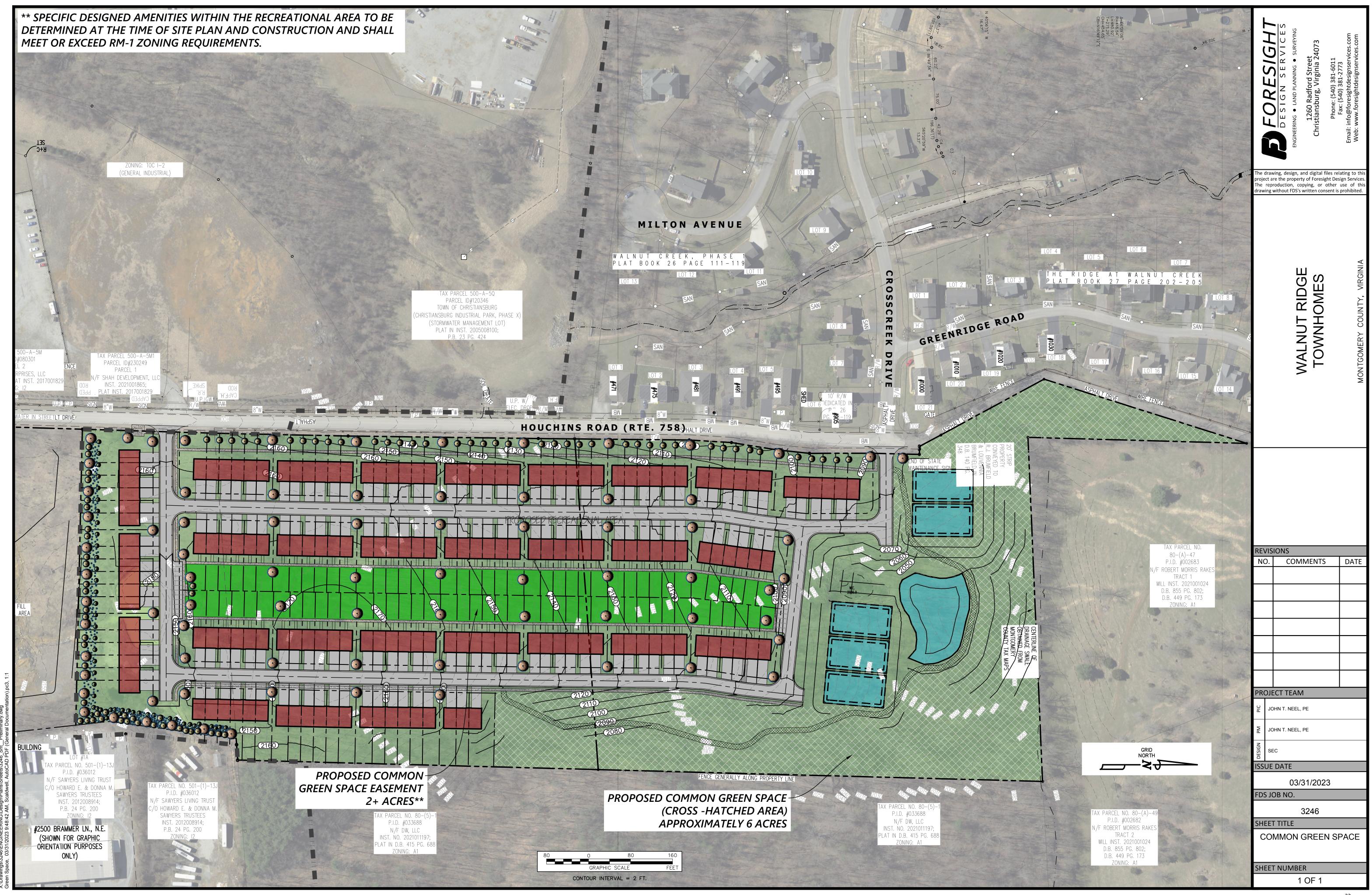


APPENDIX D: UTILITIES AND SWM PLAN



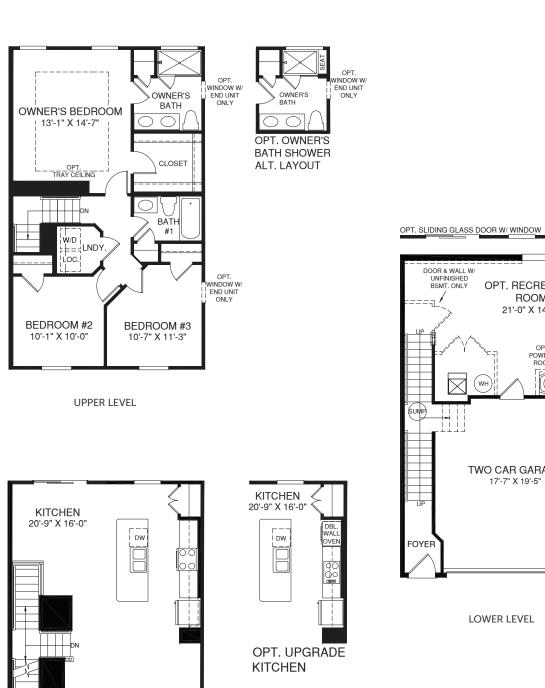
APPENDIX E: LANDSCAPE AND BUFFER PLAN

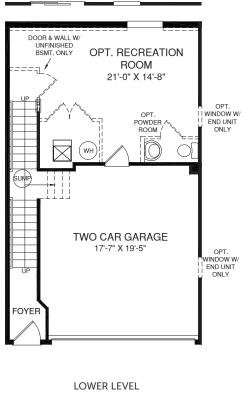




APPENDIX F: BUILDING ELEVATIONS







ELEVATIONS AND PLANS ARE FOR REPRESENTATION OF CONSTRUCTION TYPE AND IS NOT TO BE PROFFERED.

LIVING ROOM 17'-0" X 17'-6"

MAIN LEVEL





MAIN LEVEL

UPPER LEVEL

LOWER LEVEL

APPENDIX G: PSA SEWER AVAILABILITY LETTER



MONTGOMERY COUNTY PUBLIC SERVICE AUTHORITY

Government Center
Suite 2I
755 Roanoke Street
Christiansburg, VA 24073-3185

March 21, 2023

M. Todd King, Chairman Sherri Blevins, Vice-Chair Mary W. Biggs, Secretary-Treasurer Sara R. Bohn, Member April N. DeMotts, Member Darrell O. Sheppard, Member Steve R. Fijalkowski, Member

Charles E. Campbell PSA Director

John Neel Gay and Neel, Inc 1260 Radford Street Christiansburg, Virginia 24073

RE: Availability Letter
Residential Subdivision
Houchens Road
Parcel ID 018441,018437,014196
Sewer

Dear Mr. Neel:

Public sanitary sewer can be made available to this proposed residential development along Houchens Road, Parcel ID 018441,018437,014196.

Sanitary sewer service may be provided by a public sewer extension to a point adjacent to all units of this proposed development from the public sewer main located in the Walnut Creek Subdivision. You must verify that there is a minimum of two feet of fall from the building service elevation to the top of the sewer main. If adequate vertical separation does not exist, you will be required to install a public sewer pump station and force main per PSA standards. The owner would be required to obtain public easements for all portions of the sewer line extension in private property. The sewer facility fee is \$3,000.00 per each residential unit. There is also a \$750.00 pump station fee for each unit.

The sewer facilities must be designed to PSA standards by an engineer and approved by the PSA prior to construction. The owner would be required to obtain public easements for all portions of the sewer line extension in private property. The owner would be responsible for the cost of the sewer line extensions, public easements, highway permits, and any other associated requirements. These designs should be incorporated into the site development plans for this development and submitted to the PSA for review. Payment of twenty-five percent of the water sewer facility fees for all units of the development would be required prior to approval of the site plan.

Please be advised that all PSA sewer systems have a fixed number of available connections. Connections are reserved by payment of facility and connection fees, provided service is currently available to the subject property.

Also, be advised that this development must also meet all Montgomery County Planning and Zoning Department requirements. The availability of sanitary sewer facilities does not by itself authorize the development of this property.

This letter and stated fees are only valid to April 1, 2024.

If you have questions or need additional clarification on the above information, please contact me at 381-1997.

Sincerely,

Charles E. Campbell

rales E. Cobell

PSA Director

cc: Montgomery Co. Planning Dept.

Walnut Ridge

APPENDIX H: TRAFFIC IMPACT ANALYSIS

Traffic Impact Analysis

Houchins Road Townhomes

Montgomery County, Virginia

March 2023

Prepared for:

Foresight Design Services 1260 Radford Street Christiansburg, VA 24073



Prepared by:



4114 Legato Road	225 Reinekers Lane	1140 Connecticut Ave NW	4951 Lake Brook Drive
Suite 650	Suite 750	Suite 600	Suite 250
Fairfax, VA 22033	Alexandria, VA 22314	Washington, DC 20036	Glen Allen, VA 23060
T 703.787.9595	T 703.721.3044	T 202.296.8625	T 804.362.0578

www.goroveslade.com

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TRAFFIC IMPACT ANALYSIS

To: Jesse Miller, P.E.

VDOT

Brea Hopkins Cc:

Montgomery County

John Neel, P.E.

Foresight Design Services

From:

Michael Bailey, P.E., RSP1

Date:

March 2023

Subject: Houchins Road Townhomes - Montgomery County, VA

Introduction

Shah Development is proposing to construct a townhome neighborhood with 171 single-family attached homes on the east side of Houchins Road. The proposed access plan includes two full-movement driveways on Houchins Road. The site is expected to be built-out by 2027.

This Traffic Impact Analysis (TIA) was developed in accordance with Virginia Department of Transportation (VDOT), Montgomery County TIA guidelines, and our TIA scope meeting. A copy of the TIA scoping document is included in the Appendix.

Scope of the Analysis

The objective of this analysis is to identify potential impacts to the transportation network due to the proposed convenience store. Based on the TIA scoping meeting, the following scenarios were analyzed:

- Existing (2023) Conditions
- No-Build (2027) Conditions
- Build (2027) Conditions

The weekday AM and PM peak hours were studied for the following intersections:

- Roanoke Street (U.S. 11 / U.S. 460 BUS) at Houchins Road / Bristol Drive
- Houchins Road at South Site Driveway
- Houchins Road at Crosscreek Drive / North Site Driveway

Figure 1 shows the site location and study intersections and the site plan is shown in Figure 2.



Figure 1: Site Location and Study Intersections



Figure 2: Preliminary Site Plan (Prepared by Gress Engineering for informational purposes only)

Existing Conditions

Existing Roadway Network

A description of the major roadways within the study area is shown in Table 1 and the existing lane configuration is shown in Figure 3.

Table 1: Existing Roadway Network

Roadway	RTE#	VDOT Classification	Legal/Design Speed Limit (mph)	AADT* (vpd)
Roanoke Street	U.S. 11 / U.S. 460 BUS	Principal Arterial	35 mph	15,000
Houchins Road	758	Local Roadway	25 mph	860
Bristol Drive	n/a	Local Roadway	25 mph	n/a
Crosscreek Drive	n/a	Local Roadway	25 mph	760
* VDOT 2021 ADT Traffic Data				

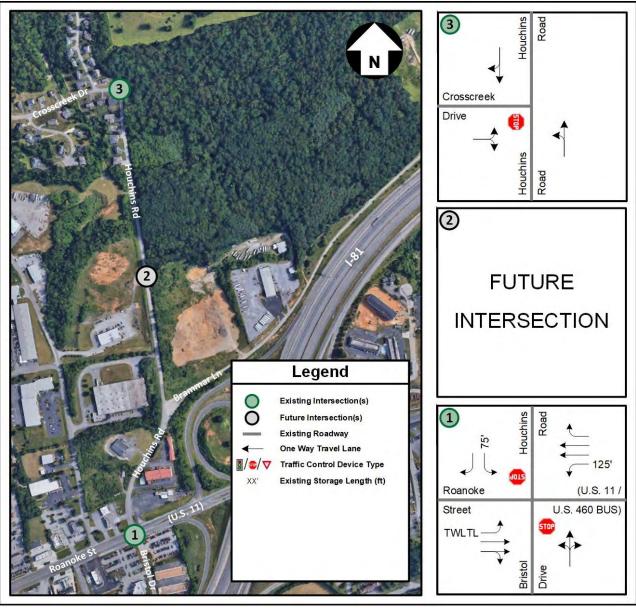


Figure 3: Existing Lane Configuration

Existing (2023) Traffic Volumes

The weekday AM peak hour (7:00 to 9:00 AM) and PM (4:00 to 6:00 PM) turning movement counts were conducted by Burns Services Inc. during the week of November 28th at the following intersections:

- Roanoke Street (U.S. 11 / U.S. 460 BUS) at Houchins Road / Bristol Drive
- Houchins Road at Crosscreek Drive / North Site Driveway

Through movements were increased to balance traffic volumes between intersections. The peak hour traffic volumes are shown in Figure 4 and the count data is included in the Appendix.

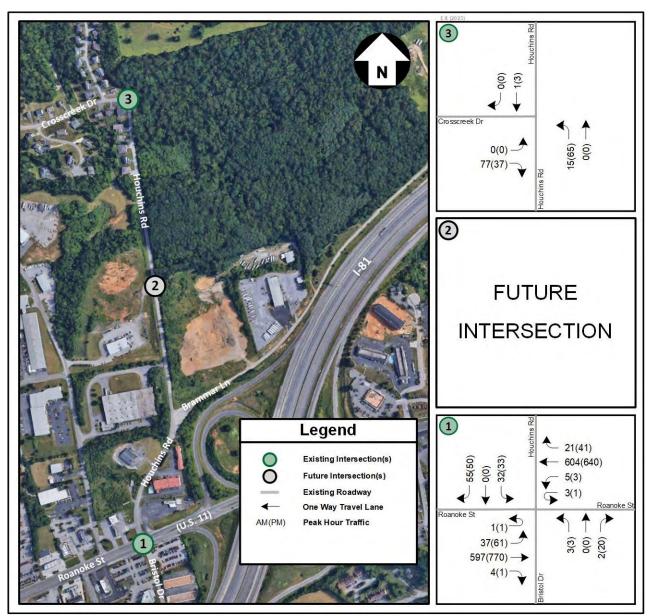


Figure 4: Existing (2023) Peak Hour Traffic Volumes

No-Build Conditions

Background Improvements

Based on the scoping meeting, no background improvements were included in this analysis.

Background Developments

Based on the scoping meeting, one approved development was identified within the vicinity of the site to be included in this analysis. The existing Walnut Creek neighborhood, located along Crosscreek Drive, is approved for the development of an additional 22 single-family detached homes and 145 single-family attached units. The anticipated development traffic is included under No-Build (2027) conditions, and the trip generation is shown in Table 2. Figure 5 shows the anticipated site trip volumes, which were based on the following regional distribution:

- 75% to / from the west on Roanoke Street
- 25% to / from the east on Roanoke Street

Table 2: ITE Trip Generation - Walnut Creek Residential - Typical Weekday - 11th Edition

	ITE					Weekd	lay		
Land Use	ITE Code	Size Units	Al	Л Peak H	our	PN	1 Peak Ho	our	Weekday
	Oode		In	Out	Total	ln	Out	Total	Total
Single-Family Detached Housing	210	22 d.u.	5	14	19	15	9	24	252
Single-Family Attached Housing	215	145 d.u.	17	53	70	49	34	83	1,054
Total Site Trips			22	67	89	64	43	107	1,306

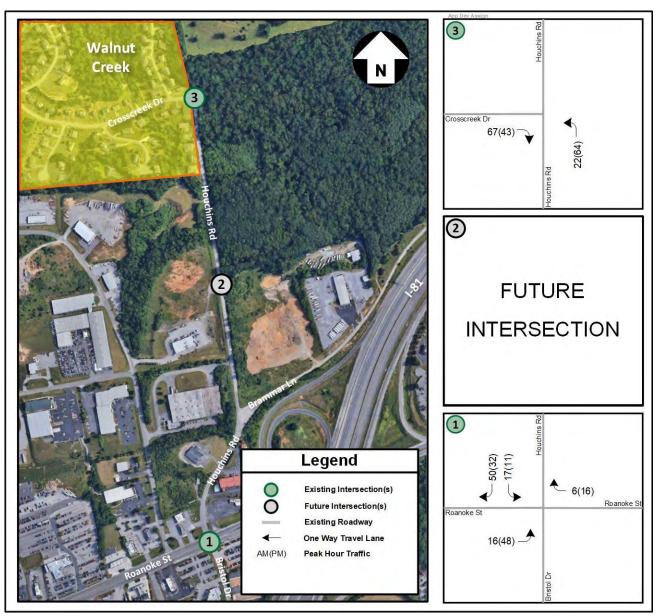


Figure 5: Walnut Creek Site Trips

Regional Traffic Growth

Future traffic volumes were estimated by increasing the existing traffic volumes to the build-out year using an annual growth rate and adding the trips generated by the background development. The no-build peak hour traffic volumes were estimated by applying an annual background growth rate of 1.0% for four years. Figure 6 shows the No-Build (2027) peak hour traffic volumes.

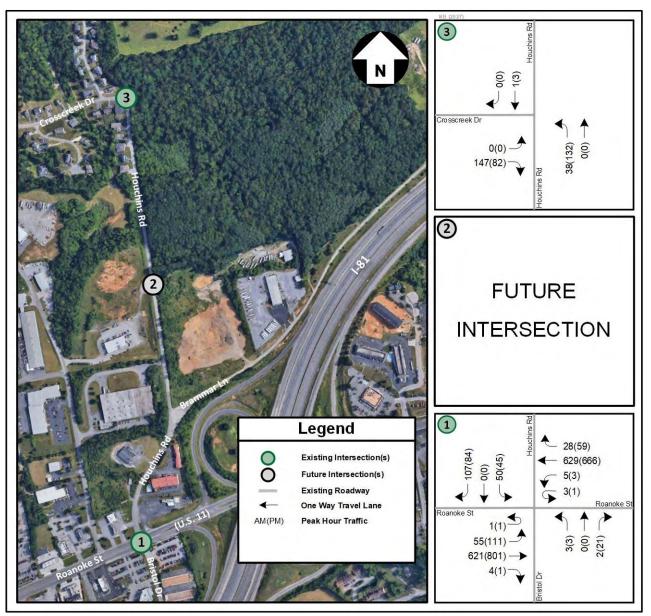


Figure 6: No-Build (2027) Peak Hour Traffic Volumes

Build Conditions

Site Generated Trips

Table 3 shows the trip potential of the proposed store based on the 11th Edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual.

Table 3: ITE Trip Generation – Houchins Road Townhomes – Typical Weekday – 11th Edition

						Weekd	ау		
Land Use	ITE Code	Size Units	Al	M Peak Ho	our	PN	1 Peak H	our	Weekday
	Code		In	Out	Total	ln	Out	Total	Total
Single-Family Attached Housing	215	171 d.u.	21	62	83	58	41	99	1,254

Primary Site Trip Distribution

The site trip distribution was based on existing traffic patterns. The following regional distribution was applied to the site trips:

- 75% to / from the west on Roanoke Street
- 25% to / from the east on Roanoke Street

Figure 7 shows the regional site trip distribution and Figure 8 site trip assignment.

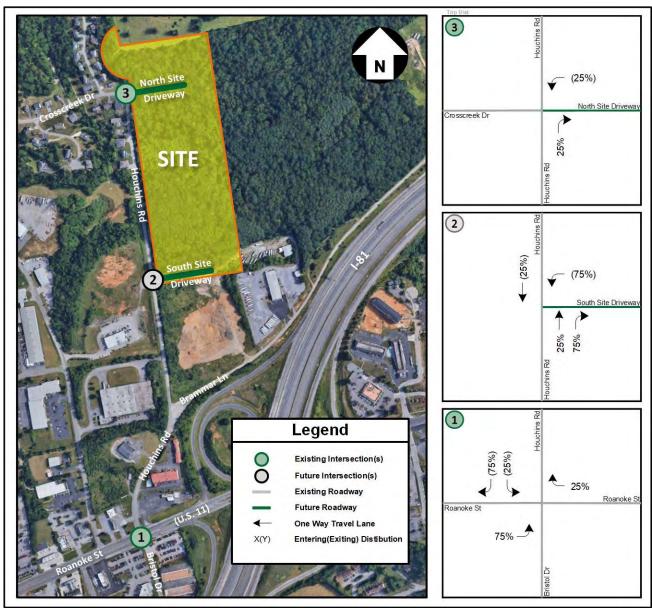


Figure 7: Primary Site Trip Distribution

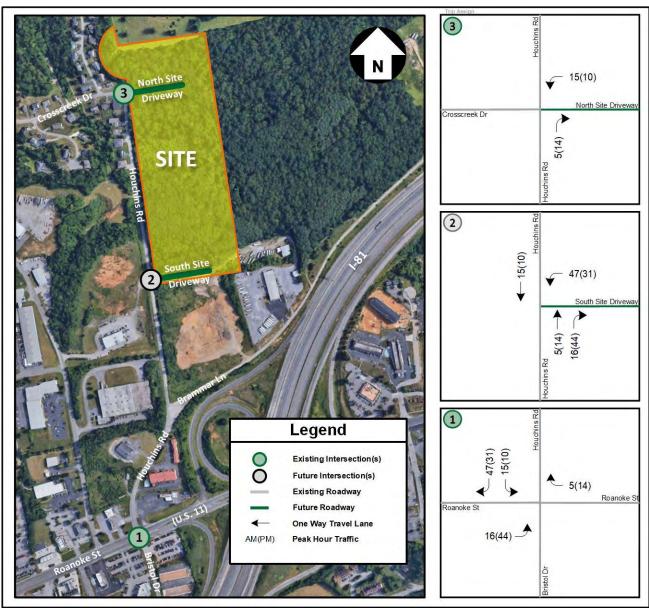


Figure 8: Primary Site Trip Assignment

Build (2027) Traffic Volumes

The build traffic volumes were estimated by adding the no-build traffic volumes (Figure 6) and the site trip assignment (Figure 8). Figure 9 shows the Build (2027) peak hour traffic volumes.

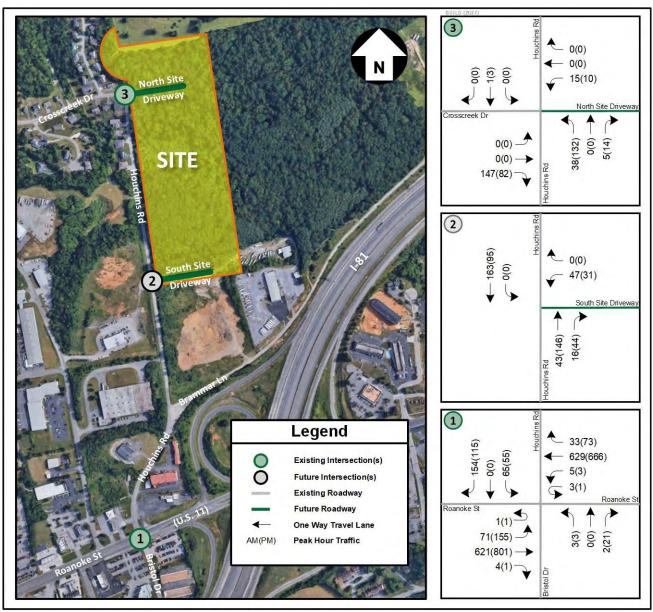


Figure 9: Build (2027) Peak Hour Traffic Volumes

Turn Lane Warrant Analysis

The need for turn lanes was evaluated at the site driveways under Build (2027) conditions. The results of the turn lane warrant analysis are summarized in Table 4.

Table 4: Right-Turn Lane Warrant Analysis (2-Lane)

Study Scenario	Approach Volume	Right Turn Volume	Minimum Right Turn Taper Threshold	Minimum Right Turn Full Lane Threshold	Treatment
INT 2 - NBR - Build 2027 AM Peak	59	16	64	110	Not Warranted
INT 2 - NBR - Build 2027 PM Peak	190	44	51	93	Not Warranted
INT 3 - NBR - Build 2027 AM Peak	43	5	66	112	Not Warranted
INT 3 - NBR - Build 2027 PM Peak	146	14	55	99	Not Warranted

Based on the turn lane warrant analysis, no turn lanes are warranted at the proposed site driveways.

Capacity Analysis

Capacity analysis was performed at the study intersections during the weekday AM and PM peak hours under all analysis scenarios. Synchro, Version 11 was used to analyze the study intersections based on the Highway Capacity Manual (HCM methodology and includes level of service (LOS), delay, and queue lengths for the turning movements analyzed. SimTraffic queues were based on the maximum of an average of 10 microsimulation runs. The queueing analysis results are summarized in the tables below and the Synchro / SimTraffic output reports are included in the appendix. A future peak hour factor (PHF) of 0.92 was used only if the existing PHF was less than 0.92, otherwise the existing PHF was used.

For unsignalized intersections, the average delays for the minor street movements are described as short delays (less than 25 seconds), moderate delays (between 25 and 50 seconds), and long delays (greater than 50 seconds). It is common for side street movements to experience long delays during the peak hours at intersections with major thoroughfares.

Table 5 shows the LOS, average delays, and queue lengths for the signalized intersection of Roanoke Street (U.S. 11 / U.S. 460 BUS) at Houchins Road / Bristol Drive.

Table 5: Level-of-Service Summary for Roanoke Street at Houchins Road / Bristol Drive

				AM	Peak Hour			PM Peak Hou	r	
Scenario	Intersection (Movement)	Effective Storage Length (ft.) [1]	LOS	Delay (sec/veh)	95th % Queue (ft.)	Ave. Max Queue (ft.) [2]	LOS	Delay (sec/veh)	95th % Queue (ft.)	Ave. Max Queue (ft.) [2]
				Synchro		SimTraffic		Synchro		SimTraffic
	Roanoke Street (U.S. 11) [E/W] at Houchins Road / Bristol Drive [N/S]								,	
	Overall Intersection (TWSC)									
	Eastbound Approach									
	Eastbound U/Left		Α	9.2	3	88	Α	9.8	8	96
Existing	Eastbound Thru/Right									
(2023)	Westbound Approach									
Conditions	Westbound U/Left	125	Α	9.8	0	28	В	10.9	0	23
Conditions	Westbound Thru									
	Westbound Right									
	Northbound Approach		С	21.2			С	17.3		
	Northbound Left/Thru/Right		С	21.2	3	32	С	17.3	8	41
	Southbound Approach		С	18.2			D	32.1		
	Southbound Left	75	D	31.0	18	58	F	63.8	40	54
	Southbound Right		В	10.8	8	71	В	11.2	8	47
	Roanoke Street (U.S. 11) [E/W] at Houchins Road / Bristol Drive [N/S]									
	Overall Intersection (TWSC)									
	Eastbound Approach									
	Eastbound U/Left		Α	9.4	5	119	В	10.2	13	118
No-Build	Eastbound Thru/Right									
	Westbound Approach									
(2027)	Westbound U/Left	125	Α	10	0	24	В	10.9	0	27
Conditions	Westbound Thru									
	Westbound Right									
	Northbound Approach		D	25.5			С	19.9		
	Northbound Left/Thru/Right		D	25.5	3	2	С	19.9	8	41
	Southbound Approach		С	21.6			F	50.4		
	Southbound Left	75	E	43.3	38	30	F	122.8	75	70
	Southbound Right		В	11.5	15	63	В	11.6	13	128
	Roanoke Street (U.S. 11) [E/W] at Houchins Road / Bristol Drive [N/S]									
	Overall Intersection (TWSC)									
	Eastbound Approach									
	Eastbound U/Left		Α	9.5	8	152	В	10.6	20	128
Build	Eastbound Thru/Right									
(2027)	Westbound Approach									
Conditions	Westbound U/Left	125	Α	10	0	25	В	10.9	0	20
Conditions	Westbound Thru						1			
	Westbound Right									
	Northbound Approach		D	29.2			С	24.1		
	Northbound Left/Thru/Right		D	29.2	3	34	С	24.1	10	2
	Southbound Approach		D	25.5			F	98.6		
	Southbound Left	75	F	57	60	74	F	279.6	128	7
	Southbound Right	1	В	12.2	25	172	В	12.1	18	47

Capacity analysis shows that the minor street left-turn movement currently operates with moderate delays during the AM peak hour and with long delays during the PM peak hour. Under no-build conditions, the minor street left-turn movement is expected to continue operating with moderate delays during the AM peak hour and with long delays during the PM peak hour. Under build conditions, the minor street left-turn movement is expected to operate with long delays during the AM and PM peak hours. It is common for side street movements and left turns to experience long delays during the peak hours at intersections with major thoroughfares. No improvements are warranted or recommended at this intersection upon build-out of the proposed site.

Table 6 shows the LOS, average delays, and queue lengths for the unsignalized intersection of Houchins Road at the proposed South Site Driveway.

Table 6: Level-of-Service Summary for Houchins Road at South Site Driveway

			AM I	Peak Hour			PM Peak Hou	r	
		LOS	Delay	95th %	Ave. Max	LOS	Delay	95th %	Ave. Max
Scenario	Intersection (Movement)		(sec/veh)	Queue (ft.)	Queue (ft.) [2]		(sec/veh)	Queue (ft.)	Queue (ft.) [2]
			Synchro		SimTraffic		Synchro		SimTraffic
	Houchins Road [N/S] at South							•	
	Site Driveway [E/W]								
	Overall Intersection (TWSC)								
Build	Westbound Approach	В	10.1			В	10.4		
(2027)	Westbound Left/Right	В	10.1	5	56	В	10.4	5	57
Conditions	Northbound Approach								
	Northbound Thru/Right								
	Southbound Approach								
	Southbound Left/Thru								

Capacity analysis shows that under build conditions, the minor street left-turn movement is expected to operate with short delays and queue lengths of three vehicles or less during the AM and PM peak hours. No improvements are warranted or recommended at the build-out of the proposed site.

Table 7 shows the LOS, average delays, and queue lengths for the unsignalized intersection of Houchins Road at Crosscreek Drive / North Site Driveway.

Table 7: Level-of-Service Summary for Houchins Road at Crosscreek Drive / North Site Driveway

	ver-or-service summary to			Peak Hour			PM Peak Hou		
Scenario	Intersection (Movement)	LOS	Delay (sec/veh)	95th % Queue (ft.)	Ave. Max Queue (ft.) ^[2]	LOS	Delay (sec/veh)	95th % Queue (ft.)	Ave. Max Queue (ft.) ^[2]
			Synchro		SimTraffic		Synchro		SimTraffic
	Houchins Road [N/S] at Crosscreek Drive [E/W] Overall Intersection (TWSC)								
Existing	Eastbound Approach								
(2023)	Eastbound Left/Right	Α	8.7	8	47	Α	8.5	3	31
Conditions	Northbound Approach								
	Northbound Left/Thru	Α	7.2	0	0	Α	7.3	3	0
	Southbound Approach								
	Southbound Thru/Right								
	Houchins Road [N/S] at Crosscreek Drive [E/W] Overall Intersection (TWSC)								
No-Build	Eastbound Approach								
(2027)	Eastbound Left/Right	Α	8.9	13	51	Α	8.6	8	47
Conditions	Northbound Approach								
	Northbound Left/Thru	Α	7.3	3	0	Α	7.4	8	3
	Southbound Approach								
	Southbound Thru/Right								
	Houchins Road [N/S] at Crosscreek Drive / North Site Driveway [E/W] Overall Intersection (TWSC)								
	Eastbound Approach	Α	8.9			Α	8.6		
Build	Eastbound Left/Thru/Right	Α	8.9	13	70	Α	8.6	8	53
(2027) Conditions	Westbound Approach	В	10.5			В	12.0		
Conditions	Westbound Left/Thru/Right	В	10.5	3	29	В	12.0	3	33
	Northbound Approach								
	Northbound Left/Thru/Right	Α	7.3	3	0	Α	7.4	8	6
	Southbound Approach								
	Southbound Left/Thru/Right								

Capacity analysis shows that the minor approach currently operates with short delays and queue lengths of two vehicles or less during the AM and PM peak hours. Under both no-build and build conditions, the minor approaches are expected to continue operating with short delays and queue lengths of three vehicles or less during the AM and PM peak hours. No improvements are warranted or recommended at the build-out of the proposed site.

Summary and Conclusions

Based on the results of the analysis, all intersections are projected to operate with acceptable delay and queueing upon completion of the proposed townhome neighborhood. No improvements are warranted or recommended at build-out of this site. Figure 10 shows the recommended lane configuration.

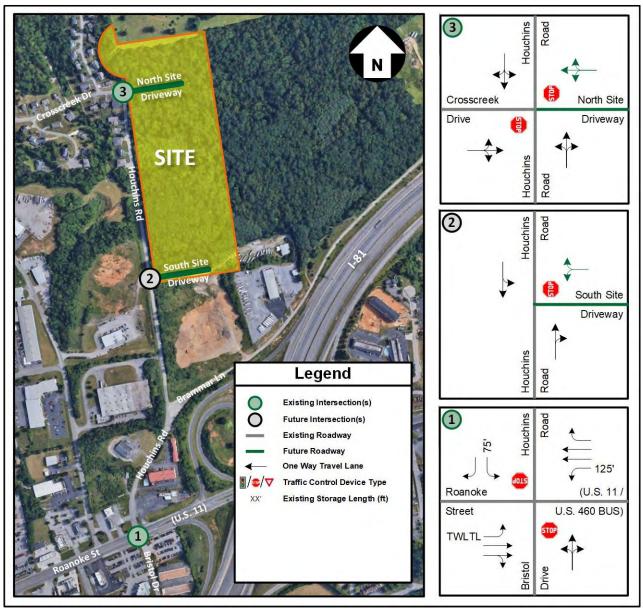


Figure 10: Recommended Lane Configuration

TECHNICAL APPENDIX

TECHNICAL APPENDIX

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APPENDIX F

Turn Lane Warrant Assessment

APPENDIX G

VDOT Pre-Scope Form

APPENDIX A

Turning Movement Count Sheets



Site Code:

Start Date : 1/1/2023

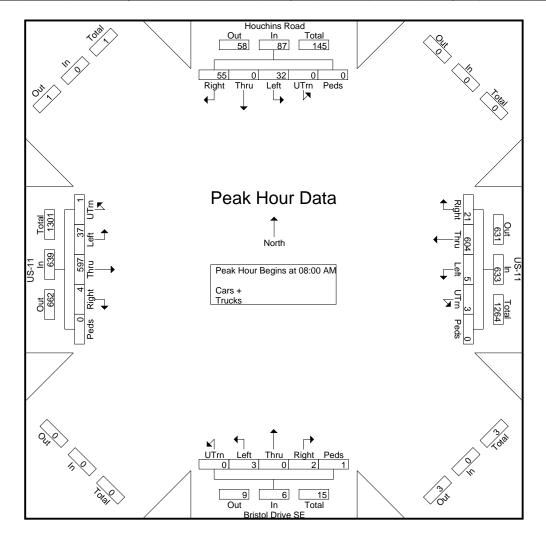
										Grou	ps Pr	inted-	Cars ·	+ - Tr	ucks										
		Н	ouchi	ns Ro	oad				US	S-11				Ві	ristol I	Drive	SE				US	3-11			
			South	boun	d				West	boun	d				North	boun	d				East	bound	1		
Start Time	Right	Thru	Left	UTrn	Peds	App. Total	Right	Thru	Left	UTrn	Peds	App. Total	Right	Thru	Left	UTrn	Peds	App. Total	Right	Thru	Left	UTrn	Peds	App. Total	Int. Total
07:00 AM	14	0	7	0	0	21	5	134	2	0	0	141	0	0	0	0	0	0	1	81	3	0	0	85	247
07:15 AM	16	0	14	0	0	30	7	172	0	0	0	179	1	0	0	0	0	1	1	86	8	0	0	95	305
07:30 AM	18	0	13	0	0	31	4	183	1	0	0	188	0	0	3	0	0	3	5	84	3	0	0	92	314
07:45 AM	17	0	6	0	0	23	5	185	5	0	0	195	1	1	2	0	0	4	4	98	9	0	0	111	333
Total	65	0	40	0	0	105	21	674	8	0	0	703	2	1	5	0	0	8	11	349	23	0	0	383	1199
08:00 AM	10	0	11	0	0	21	4	152	0	0	0	156	0	0	0	0	0	0	0	122	9	1	0	132	309
08:15 AM	15	0	11	0	0	26	7	148	1	1	0	157	0	0	0	0	1	1	0	149	12	0	0	161	345
08:30 AM	13	0	5	0	0	18	6	167	2	1	0	176	2	0	2	0	0	4	2	144	8	0	0	154	352
08:45 AM	17	0	5	0	0	22	4	137	2	1	0	144	0	0	1	0	0	1	2	182	8	0	0	192	359
Total	55	0	32	0	0	87	21	604	5	3	0	633	2	0	3	0	1	6	4	597	37	1	0	639	1365
Grand Total	120	0	72	0	0	192	42	1278	13	3	0	1336	4	1	8	0	1	14	15	946	60	1	0	1022	2564
Apprch %	62.5	0	37.5	0	0		3.1	95.7	1	0.2	0		28.6	7.1	57.1	0	7.1		1.5	92.6	5.9	0.1	0		
Total %	4.7	0	2.8	0	0	7.5	1.6	49.8	0.5	0.1	0	52.1	0.2	0	0.3	0	0	0.5	0.6	36.9	2.3	0	0	39.9	
Cars +	116	0	65	0	0	181	40	1224	13	3	0	1280	4	1	8	0	1	14	15	847	57	1	0	920	2395
% Cars +	96.7	0	90.3	0	0	94.3	95.2	95.8	100	100	0	95.8	100	100	100	0	100	100	100	89.5	95	100	0	90	93.4
Trucks	4	0	7	0	0	11	2	54	0	0	0	56	0	0	0	0	0	0	0	99	3	0	0	102	169
% Trucks	3.3	0	9.7	0	0	5.7	4.8	4.2	0	0	0	4.2	0	0	0	0	0	0	0	10.5	5	0	0	10	6.6



Site Code:

Start Date : 1/1/2023

		Н	ouchi	ns Ro	oad				US	S-11				Br	istol [Drive	SE				US	S-11			
			South	boun	d				West	boun	d				North	boun	d				East	bound	ł		İ
Start Time	Right	Thru	Left	UTrn	Peds	App. Total	Right	Thru	Left	UTrn	Peds	App. Total	Right	Thru	Left	UTrn	Peds	App. Total	Right	Thru	Left	UTrn	Peds	App. Total	Int. Total
Peak Hour	Analy	ysis F	rom (07:00	AM to	o 08:4	5 AM	- Pea	k 1 o	f 1															
Peak Hour	for E	ntire	Inters	ection	n Beg	ins at	08:00	AM																	
08:00 AM	10	0	11	0	0	21	4	152	0	0	0	156	0	0	0	0	0	0	0	122	9	1	0	132	309
08:15 AM	15	0	11	0	0	26	7	148	1	1	0	157	0	0	0	0	1	1	0	149	12	0	0	161	345
08:30 AM	13	0	5	0	0	18	6	167	2	1	0	176	2	0	2	0	0	4	2	144	8	0	0	154	352
08:45 AM	17	0	5	0	0	22	4	137	2	1	0	144	0	0	1	0	0	1	2	182	8	0	0	192	359
Total Volume	55	0	32	0	0	87	21	604	5	3	0	633	2	0	3	0	1	6	4	597	37	1	0	639	1365
% App. Total	63.2	0	36.8	0	0		3.3	95.4	8.0	0.5	0		33.3	0	50	0	16.7		0.6	93.4	5.8	0.2	0		
PHF	.809	.000	.727	.000	.000	.837	.750	.904	.625	.750	.000	.899	.250	.000	.375	.000	.250	.375	.500	.820	.771	.250	.000	.832	.951





Site Code:

Start Date : 1/1/2023

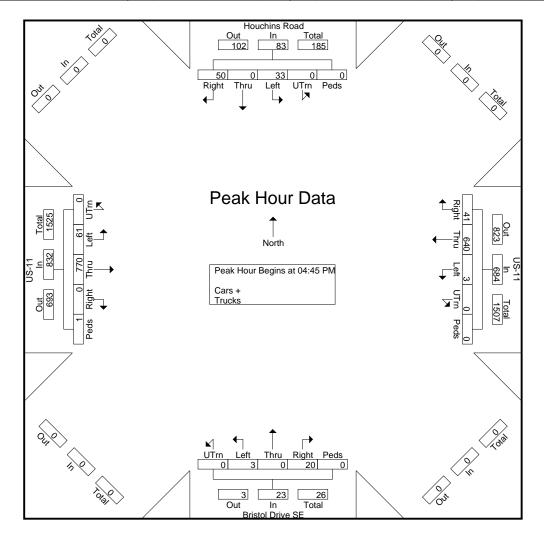
										Grou	ps Pr	inted-	Cars -	+ - Tru	ucks										
		Н	ouchi	ins Ro	oad				US	3-11				Br	istol	Drive	SE				US	3-11			ĺ
			South	bour	d				West	boun	d				North	boun	d				East	bound	t		
Start Time	Right	Thru	Left	UTrn	Peds	App. Total	Right	Thru	Left	UTrn	Peds	App. Total	Right	Thru	Left	UTrn	Peds	App. Total	Right	Thru	Left	UTrn	Peds	App. Total	Int. Total
04:00 PM	9	0	11	0	0	20	11	163	1	0	0	175	1	1	0	0	0	2	0	184	17	1	0	202	399
04:15 PM	9	0	13	0	0	22	5	154	0	0	0	159	1	0	0	0	0	1	0	188	10	0	0	198	380
04:30 PM	7	0	10	0	0	17	10	140	1	1	0	152	2	0	1	0	0	3	0	170	11	0	0	181	353
04:45 PM	9	0	4	0	0	13	10	159	0	0	0	169	5	0	0	0	0	5	0	166	15	0	0	181	368
Total	34	0	38	0	0	72	36	616	2	1	0	655	9	1	1	0	0	11	0	708	53	1	0	762	1500
05:00 PM	13	0	7	0	0	20	13	169	2	0	0	184	6	0	1	0	0	7	0	236	14	0	0	250	461
05:15 PM	21	0	6	0	0	27	8	154	1	0	0	163	6	0	1	0	0	7	0	195	19	0	1	215	412
05:30 PM	7	0	16	0	0	23	10	158	0	0	0	168	3	0	1	0	0	4	0	173	13	0	0	186	381
05:45 PM	14	0	4	0	0	18	8	139	1	1	0	149	0	0	1	0	0	1	0	147	18	0	0	165	333
Total	55	0	33	0	0	88	39	620	4	1	0	664	15	0	4	0	0	19	0	751	64	0	1	816	1587
																			'						
Grand Total	89	0	71	0	0	160	75	1236	6	2	0	1319	24	1	5	0	0	30	0	1459	117	1	1	1578	3087
Apprch %	55.6	0	44.4	0	0		5.7	93.7	0.5	0.2	0		80	3.3	16.7	0	0		0	92.5	7.4	0.1	0.1		l
Total %	2.9	0	2.3	0	0	5.2	2.4	40	0.2	0.1	0	42.7	0.8	0	0.2	0	0	1	0	47.3	3.8	0	0	51.1	l
Cars +	87	0	70	0	0	157	72	1217	6	2	0	1297	24	1	5	0	0	30	0	1434	116	1	1	1552	3036
% Cars +	97.8	0	98.6	0	Ō	98.1	96	98.5	100	100	0	98.3	100	100	100	0	Ō	100	0	98.3	99.1	100	100	98.4	98.3
Trucks	2	0	1	0	0	3	3	19	0	0	0	22	0	0	0	0	0	0	0	25	1	0	0	26	51
% Trucks	2.2	0	1.4	0	Ō	1.9	4	1.5	Ō	Ō	Ō	1.7	Ō	Ō	Ō	Ō	Ō	0	Ō	1.7	0.9	0	Ō	1.6	1.7



Site Code:

Start Date : 1/1/2023

		Н	ouchi	ns Ro	oad				US	S-11				Br	istol [Drive	SE				US	S-11			
			South	boun	ıd				West	boun	d				North	boun	d				East	bound	b		
Start Time	Right	Thru	Left	UTrn	Peds	App. Total	Right	Thru	Left	UTrn	Peds	App. Total	Right	Thru	Left	UTrn	Peds	App. Total	Right	Thru	Left	UTrn	Peds	App. Total	Int. Total
Peak Hour	Anal	sis F	rom (04:00	PM t	o 05:4	5 PM	- Pea	k 1 o	f 1															
Peak Hour	for E	ntire	Inters	ection	n Beg	ins at	04:45	PM																	
04:45 PM	9	0	4	0	0	13	10	159	0	0	0	169	5	0	0	0	0	5	0	166	15	0	0	181	368
05:00 PM	13	0	7	0	0	20	13	169	2	0	0	184	6	0	1	0	0	7	0	236	14	0	0	250	461
05:15 PM	21	0	6	0	0	27	8	154	1	0	0	163	6	0	1	0	0	7	0	195	19	0	1	215	412
05:30 PM	7	0	16	0	0	23	10	158	0	0	0	168	3	0	1	0	0	4	0	173	13	0	0	186	381
Total Volume	50	0	33	0	0	83	41	640	3	0	0	684	20	0	3	0	0	23	0	770	61	0	1	832	1622
% App. Total	60.2	0	39.8	0	0		6	93.6	0.4	0	0		87	0	13	0	0		0	92.5	7.3	0	0.1		
PHF	.595	.000	.516	.000	.000	.769	.788	.947	.375	.000	.000	.929	.833	.000	.750	.000	.000	.821	.000	.816	.803	.000	.250	.832	.880





Site Code:

Start Date : 2/22/2023

Page No : 1

Groups Printed- Cars + - Trucks

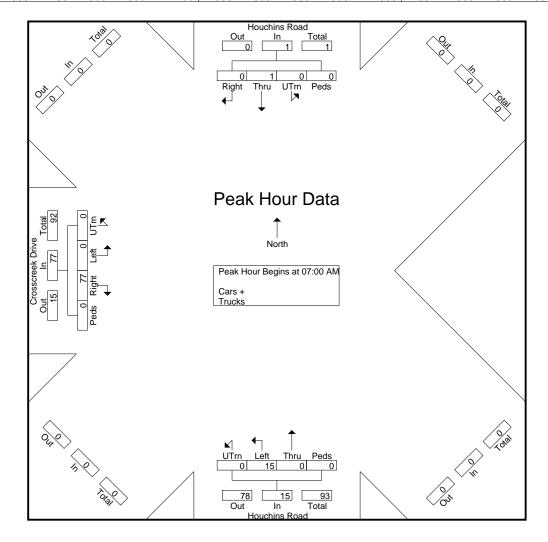
		Hoi	uchins F	Road		Group		<u>d- Cars</u> uchins F		,KS		Cros	sscreek	Drive		
		S	outhbou	ınd			N	orthbou	ınd				astbou			
Start Time	Right	Thru	UTrn	Peds	App. Total	Thru	Left	UTrn	Peds	App. Total	Right	Left	UTrn	Peds	App. Total	Int. Total
07:00 AM	0	0	0	0	0	0	1	0	0	1	17	0	0	0	17	18
07:15 AM	0	0	0	0	0	0	7	0	0	7	23	0	0	0	23	30
07:30 AM	0	0	0	0	0	0	1	0	0	1	21	0	0	0	21	22
07:45 AM	0	1_	0_	0	1	0	6	0	0	6	16	0	0	0	16	23
Total	0	1	0	0	1	0	15	0	0	15	77	0	0	0	77	93
1																
08:00 AM	0	0	0	0	0	0	3	0	0	3	12	0	0	0	12	15
08:15 AM	0	0	0	0	0	0	7	0	0	7	19	0	1	0	20	27
08:30 AM	0	0	0	0	0	0	4	0	0	4	8	0	0	0	8	12
08:45 AM	0	0	0	0	0	0	3	0	0	3	13	0	0	0	13	16
Total	0	0	0	0	0	0	17	0	0	17	52	0	1	0	53	70
- 1																
Grand Total	0	1	0	0	1	0	32	0	0	32	129	0	1	0	130	163
Apprch %	0	100	0	0		0	100	0	0		99.2	0	8.0	0		
Total %	0	0.6	0	0	0.6	0	19.6	0_	0	19.6	79.1	0	0.6	0	79.8	
Cars +	0	1	0	0	1	0	30	0	0	30	126	0	1	0	127	158
% Cars +	0	100	0	0	100	0	93.8	0	0	93.8	97.7	0	100	0	97.7	96.9
Trucks	0	0	0	0	0	0	2	0	0	2	3	0	0	0	3	5
% Trucks	0	0	0	0	0	0	6.2	0	0	6.2	2.3	0	0	0	2.3	3.1



Site Code:

Start Date : 2/22/2023

		Ho	uchins F	Road			Ho	uchins R	Road											
		S	outhbou	nd			N	lorthbou	nd			E	Eastbour							
Start Time	Right	Thru	UTrn	Peds	App. Total	Thru	Left	UTrn	Peds	App. Total	Right	Left	UTrn	Peds	App. Total	Int. Total				
Peak Hour Anal	ysis Fron	n 07:00	AM to 0	8:45 AN	/I - Peak 1	of 1														
Peak Hour for E	ntire Inte	rsection	n Begins	at 07:0	0 AM															
07:00 AM	0	0	0	0	0	0	1	0	0	1	17	0	0	0	17	18				
07:15 AM	0	0	0	0	0	0	7	0	0	7	23	0	0	0	23	30				
07:30 AM	0	0	0	0	0	0	1	0	0	1	21	0	0	0	21	22				
07:45 AM	0	1	0	0	1	0	6	0	0	6	16	0	0	0	16	23				
Total Volume	0	1	0	0	1	0	15	0	0	15	77	0	0	0	77	93				
% App. Total	0	100	0	0		0	100	0	0		100	0	0	0						
PHF	.000	.250	.000	.000	.250	.000	.536	.000	.000	.536	.837	.000	.000	.000	.837	.775				





Site Code:

Start Date : 2/22/2023

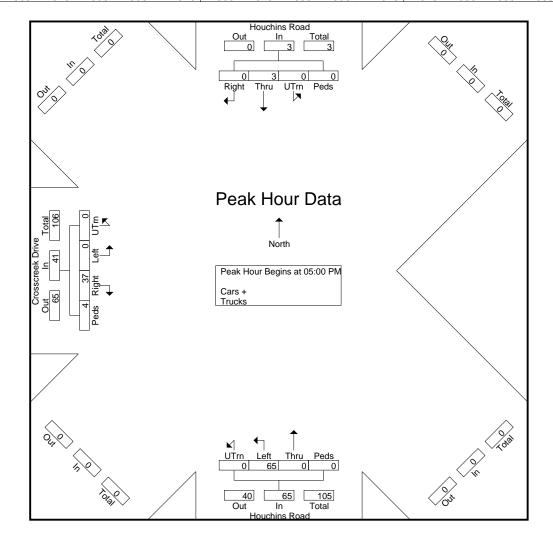
						Group	s Printe	d- Cars	+ - Truc	cks						
		Но	uchins F	Road		_	uchins F	Road			Cro	sscreek	Drive			
		S	outhbou	und			N	lorthbou	ınd				Eastbou	nd		
Start Time	Right	Thru	UTrn	Peds	App. Total	Thru	Left	UTrn	Peds	App. Total	Right	Left	UTrn	Peds	App. Total	Int. Total
04:00 PM	0	0	0	0	0	0	15	0	0	15	8	0	0	0	8	23
04:15 PM	0	0	0	0	0	0	9	0	0	9	5	0	0	0	5	14
04:30 PM	0	0	0	0	0	1	12	0	0	13	5	0	0	0	5	18
04:45 PM	0	0	0	0	0	0	19	0	0	19	7	0	0	0	7	26
Total	0	0	0	0	0	1	55	0	0	56	25	0	0	0	25	81
05:00 PM	0	2	0	0	2	0	18	0	0	18	10	0	0	0	10	30
05:15 PM	0	1	0	0	1	0	15	0	0	15	10	0	0	1	11	27
05:30 PM	0	0	0	0	0	0	12	0	0	12	10	0	0	0	10	22
05:45 PM	0	0	0	0	0	0	20	0	0	20	7	0	0	3	10	30
Total	0	3	0	0	3	0	65	0	0	65	37	0	0	4	41	109
											i					
Grand Total	0	3	0	0	3	1	120	0	0	121	62	0	0	4	66	190
Apprch %	0	100	0	0		8.0	99.2	0	0		93.9	0	0	6.1		
Total %	0	1.6	0	0	1.6	0.5	63.2	0	0	63.7	32.6	0	0	2.1	34.7	
Cars +	0	3	0	0	3	1	118	0	0	119	60	0	0	4	64	186
% Cars +	0	100	0	0	100	100	98.3	0	0	98.3	96.8	0	0	100	97	97.9
Trucks	0	0	0	0	0	0	2	0	0	2	2	0	0	0	2	4
% Trucks	0	0	0	0	0	0	1.7	0	0	1.7	3.2	0	0	0	3	2.1



Site Code:

Start Date : 2/22/2023

		Ho	uchins F	Road			Ho	uchins R	Road							
		S	outhbou	nd			N	orthbou	nd			E	astbour	nd		
Start Time	Right	Thru	UTrn	Peds	App. Total	Thru	Left	UTrn	Peds	App. Total	Right	Left	UTrn	Peds	App. Total	Int. Total
Peak Hour Anal	ysis Fron	n 04:00	PM to 0	5:45 PN	/I - Peak 1	of 1					_					
Peak Hour for E	ntire Inte	ersection	n Begins	at 05:0	0 PM											
05:00 PM	0	2	0	0	2	0	18	0	0	18	10	0	0	0	10	30
05:15 PM	0	1	0	0	1	0	15	0	0	15	10	0	0	1	11	27
05:30 PM	0	0	0	0	0	0	12	0	0	12	10	0	0	0	10	22
05:45 PM	0	0	0	0	0	0	20	0	0	20	7	0	0	3	10	30
Total Volume	0	3	0	0	3	0	65	0	0	65	37	0	0	4	41	109
% App. Total	0	100	0	0		0	100	0	0		90.2	0	0	9.8		
PHF	.000	.375	.000	.000	.375	.000	.813	.000	.000	.813	.925	.000	.000	.333	.932	.908



APPENDIX B

Level of Service Definitions

TECHNICAL MEMORANDUM

Subject: Level of Service Definitions

Introduction

The purpose of this memorandum is to define the level of service (LOS) metric that commonly used as a measure of effectiveness (MOE) for traffic operations.

All capacity analyses are based on the procedures specified by the Transportation Research Board's (TRB) <u>Highway Capacity Manual</u> (HCM), which is currently on its sixth edition. Level of service ranges from A to F. A brief description of each level of service for signalized and unsignalized intersections is provided below.

Signalized Intersections

Level of service is based upon the traffic volume present in each lane on the roadway, the capacity of each lane at the intersection and the delay associated with each directional movement. The levels of service for signalized intersections are defined below:

- <u>Level of Service A</u> describes operations with very low average delay per vehicle, i.e., less than 10.0 seconds. This
 occurs when progression is extremely favorable, and most vehicles arrive during the green phase. Most vehicles do
 not stop. Short signal cycle lengths may also contribute to low delay.
- <u>Level of Service B</u> describes operations with average delay in the range of 10.1 to 20.0 seconds per vehicle. This generally occurs with good progression and/or short cycle lengths. More vehicles stop than for LOS A, causing higher levels of average delay.
- Level of Service C describes operations with delay in the range of 20.1 to 35.0 seconds per vehicle. These higher delays may result from fair progression and/or longer cycle lengths. Individual cycle failures may begin to appear at this level. The number of vehicles stopping is significant at this level although many still pass through the intersection without stopping. This is generally considered the lower end of the range of the acceptable level of service in rural areas.
- Level of Service D describes operations with delay in the range of 35.1 to 55.0 seconds per vehicle. At LOS D, the influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavorable progression, long cycle lengths, and/or high traffic volumes as compared to the roadway capacity. Many vehicles are required to stop and the number of vehicles that do not have to stop declines. Individual signal cycle failures, where all waiting vehicles do not clear the intersection during a single green time, are noticeable. This is generally considered the lower end of the range of the acceptable level of service in urban areas.
- <u>Level of Service E</u> describes operations with delay in the range of 55.1 to 80.0 seconds per vehicle. These higher
 delay values generally indicate poor progression, long cycle lengths, and high traffic volumes. Individual cycle failures
 are frequent occurrences. LOS E has been set as the limit of acceptable conditions.
- <u>Level of Service F</u> describes operations with average delay in excess of 80.0 seconds per vehicle. This is considered to be unacceptable to most drivers. This condition often occurs with over-saturation, i.e., when traffic arrives at a flow rate that exceeds the capacity of the intersection. It may also occur at high volumes with many individual cycle failures. Poor progression and long cycle lengths may also contribute to such delays.

Unsignalized Intersections

At an unsignalized intersection, the major street through traffic and right-turns are assumed to operate unimpeded and therefore receive no level of service rating. The level of service for the minor street and the major street left-turn traffic is dependent on the volume and capacity of the available lanes, and, the number and frequency of acceptable gaps in the major street traffic to make a conflicting turn. The level of service grade is provided for each conflicting movement at an unsignalized intersection and is based on the total average delay experienced by each vehicle. The delay includes the time it takes a vehicle to move from the back of a queue through the intersection.

The unsignalized intersection level of service analysis does not account for variations in driver behavior or the effects of nearby traffic signals. Therefore, the results from this analysis usually indicate worse levels of service than may be experienced in the field. The unsignalized intersection level of service descriptions are provided below:

- <u>Level of Service A</u> describes operations where there is very little to no conflicting traffic for a minor side street movement, i.e., an average total delay of less than 10.0 seconds per vehicle.
- Level of Service B describes operations with average total delay in the range of 10.1 to 15.0 seconds per vehicle.
- Level of Service C describes operations with average total delay in the range of 15.1 to 25.0 second per vehicle.
- Level of Service D describes operations with average total delay in the range of 25.1 to 35.0 seconds per vehicle.
- Level of Service E describes operations with average total delay in the range of 35.1 to 50.0 seconds per vehicle.
- Level of Service F describes operations with average total delay of 50 seconds per vehicle. LOS F exists when there are insufficient gaps of suitable size to allow a side street demand to cross safely through or enter a major street traffic stream. This level of service is generally evident from extremely long total delays experienced by side street traffic and by queuing on the minor approaches. It is important to note that LOS F may not always result in long queues but may result in adjustments to normal driver behavior.

APPENDIX C

Intersection Capacity Analysis Results – Existing Conditions (2023)

Intersection	4.5													
Int Delay, s/veh	1.8													
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			41			Ä	^	- 7		4				7
Traffic Vol, veh/h	1	37	597	4	3	5	604	21	3	0	2	32	0	55
Future Vol, veh/h	1	37	597	4	3	5	604	21	3	0	2	32	0	55
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	-	None	-	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	125	-	0	-	-	-	75	-	0
Veh in Median Storage,	# -	-	0	-	-	-	0	-	-	0	-	-	0	-
Grade, %	-	-	0	-	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	39	628	4	3	5	636	22	3	0	2	34	0	58
Major/Minor N	1ajor1			N	Major2			N	Minor1		N	Minor2		
Conflicting Flow All	636	658	0	0	633	632	0	0	1044	1384	316	1046	-	318
Stage 1	-	-	-	-	-	-	-	-	710	710	-	652	-	-
Stage 2	_	_	_	-	_	_	_	_	334	674		394		_
Critical Hdwy	6.44	4.14	-	-	6.44	4.14	-	-	7.54	6.54	6.94	7.54	-	6.94
Critical Hdwy Stg 1	-		_	_	-	-	_	_	6.54	5.54	-	6.54		-
Critical Hdwy Stg 2	-	-	-	-	-	-	-	-	6.54	5.54	-	6.54	-	_
Follow-up Hdwy	2.52	2.22	_	_	2.52	2.22	_	_	3.52	4.02	3.32	3.52		3.32
Pot Cap-1 Maneuver	567	926	-	-	570	947	-	-	183	142	680	183	0	678
Stage 1	-		_	_		-	_	_	391	435	-	423	0	- 3, 3
Stage 2	-	-	-	-	-	-	-	-	653	452	-	602	0	_
Platoon blocked, %			_	-			-	_	500					
Mov Cap-1 Maneuver	907	907	-	-	757	757	-	-	157	131	680	172	-	678
Mov Cap-2 Maneuver	-	_	_	-	-	-	-	-	157	131	-	172	-	-
Stage 1	-	-	-	_	_	-	-	-	364	405	-	394	-	_
Stage 2	-	-	_	-	-	-	-	-	591	447	-	559	-	-
, 														
Approach	EB				WB				NB			SB		
	1.1				0.1				21.2			18.2		
HCM Control Delay, s HCM LOS	1.1				U. I				21.2 C			18.2 C		
TICIVI LUS									C			C		
NA!		UDI 4	ED!	EDT	ED.0	MDI	MOT	MED		CDL C				
Minor Lane/Major Mvmt		VBLn1	EBL	EBT	EBR	WBL	WBT	WBR S						
Capacity (veh/h)		227	907	-	-	757	-	-	172	678				
HCM Cartest Palace (a)		0.023		- 0 /		0.011	-			0.085				
HCM Control Delay (s)		21.2	9.2	0.6	-	9.8	-	-	31	10.8				
HCM Lane LOS		C	A	A	-	A	-	-	D	В				
HCM 95th %tile Q(veh)		0.1	0.1	-	-	0	-	-	0.7	0.3				

Intersection						
Int Delay, s/veh	8.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	₽	
Traffic Vol, veh/h	0	77	15	0	1	0
Future Vol, veh/h	0	77	15	0	1	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- -	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage			_	0	0	_
Grade, %	σ, π 0	-	-	0	0	-
Peak Hour Factor	78		78	78	78	78
		78				
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	99	19	0	1	0
Major/Minor	Minor2	1	Major1	N	/lajor2	
Conflicting Flow All	39	1	1	0	-	0
Stage 1	1	-	-	-	-	-
Stage 2	38	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	_	_	_	-	_
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy		3.318	2 218	_	_	_
Pot Cap-1 Maneuver	973	1084	1622	_	_	_
Stage 1	1022	-	1022	_	_	_
Stage 2	984	_	-			_
Platoon blocked, %	704	-	-	-	-	_
	041	1004	1/22	-		
Mov Cap-1 Maneuver	961	1084	1622	-	-	-
Mov Cap-2 Maneuver	961	-	-	-	-	-
Stage 1	1010	-	-	-	-	-
Stage 2	984	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.7		7.2		0	
HCM LOS	Α		7.2		U	
HOW LOS	Λ					
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1622	-	1084	-	-
HCM Lane V/C Ratio		0.012	-	0.091	-	-
HCM Control Delay (s)		7.2	0	8.7	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q(veh)	0	-	0.3	-	-

Intersection															
Int Delay, s/veh	3														
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations			414			7.4	^	7		4		ř		7	
Traffic Vol, veh/h	1	61	770	1	1	3	640	41	3	0	20	33	0	50	
Future Vol, veh/h	1	61	770	1	1	3	640	41	3	0	20	33	0	50	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	-	None	-	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	125	-	0	-	-	-	75	-	0	
Veh in Median Storage,	# -	-	0	-	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	-	0	-	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88	88	88	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	1	69	875	1	1	3	727	47	3	0	23	38	0	57	
Major/Minor M	lajor1			N	Major2			١	/linor1		N	Minor2			
Conflicting Flow All	727	774	0	0	876	876	0	0	1388	1798	438	1313	-	364	
Stage 1	-	-	-	-	-	-	-	-	1016	1016	-	735	-	-	
Stage 2	-	-	-	-	-	-	-	-	372	782	-	578	-	-	
Critical Hdwy	6.44	4.14	-	-	6.44	4.14	-	-	7.54	6.54	6.94	7.54	-	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	6.54	5.54	-	6.54	-	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	-	-	6.54	5.54	-	6.54	-	-	
Follow-up Hdwy	2.52	2.22	_	-	2.52	2.22	-	-	3.52	4.02	3.32	3.52	_	3.32	
Pot Cap-1 Maneuver	497	837	-	_	399	766	-	-	102	79	567	116	0	633	
Stage 1	_		_	_	-	_	_	_	255	314	_	377	0	-	
Stage 2	-	-	_	_	-	-	-	-	621	403	_	468	0	-	
Platoon blocked, %			-	_			-	_							
Mov Cap-1 Maneuver	826	826	_	_	614	614	-	-	81	65	567	97	-	633	
Mov Cap-2 Maneuver	-	-	_	-			-	-	81	65	-	97	_	_	
Stage 1	-	-	_	-	-	_	-	-	213	262	_	315	-	-	
Stage 2	_	-	_	_	_	_	-	-	561	400	_	375	_	_	
									301	.55					
Approach	EB				WB				NB			SB			
HCM Control Delay, s	2				0.1				17.3			32.1			
HCM LOS					U. 1				C			D			
Minor Lane/Major Mvmt	ľ	NBLn1	EBL	EBT	EBR	WBL	WBT	WRR	SBI n1	SBLn2					
Capacity (veh/h)	<u> </u>	318	826	-	LDIX	614	-	- VVDIC	97	633					
HCM Lane V/C Ratio		0.082		_		0.007	_		0.387	0.09					
HCM Control Delay (s)		17.3	9.8	1.4		10.9	_		63.8	11.2					
HCM Lane LOS		17.3	7.0 A	Α	-	В	_	_	03.0 F	В					
HCM 95th %tile Q(veh)		0.3	0.3	-	_	0	_		1.6	0.3					
HOW FOUT FOUTE Q(VEH)		0.3	0.5			U	_		1.0	0.3					

Intersection						
Int Delay, s/veh	7.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	**			4	♣	
Traffic Vol, veh/h	0	37	65	0	3	0
Future Vol, veh/h	0	37	65	0	3	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storag		-	_	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	41	71	0	3	0
IVIVIIIL FIOW	U	41	/ 1	U	3	U
Major/Minor	Minor2		Major1	N	Major2	
Conflicting Flow All	145	3	3	0	-	0
Stage 1	3	-	-	-	-	-
Stage 2	142	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	_	_	-
Critical Hdwy Stg 2	5.42	_	_	_	_	_
Follow-up Hdwy	3.518			_	_	_
Pot Cap-1 Maneuver	847	1081	1619	_	_	_
Stage 1	1020	-	1017	_	_	_
Stage 2	885	_	-			_
Platoon blocked, %	003	_	_			
Mov Cap-1 Maneuver	810	1081	1619	-		-
		1001	1019	-	-	
Mov Cap-2 Maneuver			-	-		-
Stage 1	975	-	-	-	-	-
Stage 2	885	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s			7.3		0	
HCM LOS	A		7.0			
110111 200	, ,					
Minor Lane/Major Mvr	nt	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1619	-	1081	-	-
HCM Lane V/C Ratio		0.044	-	0.038	-	-
HCM Control Delay (s	5)	7.3	0	8.5	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q(veh	٦)	0.1	-	0.1	-	-
·						

APPENDIX D

Intersection Capacity Analysis Results – No-Build Conditions (2027)

Intersection															
Int Delay, s/veh	3.1														
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations			414			Į,	^	7		4		7		7	
Traffic Vol, veh/h	1	55	621	4	3	5	629	28	3		2	50	0	107	
Future Vol, veh/h	1	55	621	4	3	5	629	28	3	0	2	50	0	107	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	-	None	-	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	125	-	0	-	-	-	75	-	0	
Veh in Median Storage, #	# -	-	0	-	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	-	0	-	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95	95	95	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	1	58	654	4	3	5	662	29	3	0	2	53	0	113	
Major/Minor Ma	ajor1			N	Major2			N	/linor1		N	/linor2			
Conflicting Flow All	662	691	0	0	658	658	0	0	1121	1481	329	1123	-	331	
Stage 1	-	-	-	-	-	-	-	-	774	774	-	678	-	-	
Stage 2	-	-	-	-	-	-	-	-	347	707	-	445	-	-	
Critical Hdwy	6.44	4.14	-	-	6.44	4.14	-	-	7.54	6.54	6.94	7.54	-	6.94	
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	6.54	5.54	-	6.54	-	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	-	-	6.54	5.54	-	6.54	-	-	
Follow-up Hdwy	2.52	2.22	-	-	2.52	2.22	-	-	3.52	4.02	3.32	3.52	-	3.32	
Pot Cap-1 Maneuver	546	900	-	-	549	926	-	-	161	124	667	160	0	665	
Stage 1	-	-	-	-	-	-	-	-	357	406	-	408	0	-	
Stage 2	-	-	-	-	-	-	-	-	642	436	-	562	0	-	
Platoon blocked, %			-	-			-	-							
Mov Cap-1 Maneuver	885	885	-	-	735	735	-	-	122	110	667	145	-	665	
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	-	122	110	-	145	-	-	
Stage 1	-	-	-	-	-	-	-	-	320	363	-	365	-	-	
Stage 2	-	-	-	-	-	-	-	-	527	431	-	501	-	-	
Approach	EB				WB				NB			SB			
HCM Control Delay, s	1.5				0.1				25.5			21.6			
HCM LOS									D			С			
Minor Lane/Major Mvmt	N	IBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1	SBLn2					
Capacity (veh/h)		181	885	-	-	735	-		145	665					
HCM Lane V/C Ratio			0.065	-	-	0.011	-	-		0.169					
		25.5	9.4	0.8	_	10	_	_	43.3	11.5					
HCM Control Delay (s)		20.0	/	0.0		10			70.0	11.0					
HCM Control Delay (s) HCM Lane LOS		23.3 D	Α	Α	-	A	-	-	+3.5 E	В					

Intersection						
Int Delay, s/veh	8.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	LDIX	IVDE	4	♣	ODIT
Traffic Vol, veh/h	0	147	38	0	1	0
Future Vol, veh/h	0	147	38	0	1	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	310p	None	-		-	None
Storage Length	0	-	_	-	_	TVOITE
Veh in Median Storage		_		0	0	_
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	160	41	0	1	0
Major/Minor N	Minor2	ľ	Major1	N	/lajor2	
Conflicting Flow All	83	1	1	0	-	0
Stage 1	1	-	-	-	-	-
Stage 2	82	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	_	-
Critical Hdwy Stg 2	5.42	_	-	-	-	-
Follow-up Hdwy		3.318	2 218	_	_	_
Pot Cap-1 Maneuver	919	1084	1622	-	_	_
Stage 1	1022	-	-	_	_	_
Stage 2	941	_	_	_	_	_
Platoon blocked, %	741	_	_	_	_	_
Mov Cap-1 Maneuver	896	1084	1622	-	-	-
Mov Cap-1 Maneuver	896	1004	1022	-	-	_
	996	-	-	-	-	-
Stage 1		-	-	-	-	-
Stage 2	941	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.9		7.3		0	
HCM LOS	А					
		NDI	NDT	EDI 4	ODT	000
Minor Lane/Major Mvm	ıt	NBL	NRII	EBLn1	SBT	SBR
Capacity (veh/h)		1622	-		-	-
HCM Lane V/C Ratio		0.025	-	0.147	-	-
		7.3	0	8.9	_	-
HCM Control Delay (s)						
		7.3 A 0.1	A	A 0.5	-	-

Intersection															
Int Delay, s/veh	5.6														
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations			414			Ä	^	7		4		*		7	
Traffic Vol, veh/h	1	111	801	1	1	3	666	59	3	0	21	45	0	84	
Future Vol, veh/h	1	111	801	1	1	3	666	59	3	0	21	45	0	84	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	-	None	-	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	125	-	0	-	-	-	75	-	0	
Veh in Median Storage,	# -	-	0	-	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	-	0	-	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	1	121	871	1	1	3	724	64	3	0	23	49	0	91	
Major/Minor Major/Minor	ajor1				Major2			N	/linor1		N	/linor2			
Conflicting Flow All	724	788	0	0	872	872	0	0	1486	1912	436	1412	-	362	
Stage 1	-	-	-	-	-	-	-	-	1116	1116	-	732	_	-	
Stage 2	_	-	_	_		_	_	_	370	796	_	680	_	_	
	6.44	4.14	_	-	6.44	4.14	-	_	7.54	6.54	6.94	7.54	_	6.94	
Critical Hdwy Stg 1	-	-	_	_	-	-	_	_	6.54	5.54	-	6.54	_	-	
Critical Hdwy Stg 2	-	-	-	-	-	-	-	-	6.54	5.54	-	6.54	-	-	
Follow-up Hdwy	2.52	2.22	_	_	2.52	2.22	_	_	3.52	4.02	3.32	3.52	_	3.32	
Pot Cap-1 Maneuver	499	827	-	-	401	769	-	_	86	67	568	98	0	635	
Stage 1	-	-	-	-	_	_	-	_	221	281	-	379	0	_	
Stage 2	-	-	-	-	-	-	-	-	622	397	-	407	0	-	
Platoon blocked, %			-	-			-	-							
Mov Cap-1 Maneuver	820	820	-	-	616	616	-	-	57	47	568	73	-	635	
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	-	57	47	-	73	-	-	
Stage 1	-	-	-	-	-	-	-	-	157	200	-	270	-	-	
Stage 2	-	-	-	-	-	-	-	-	529	395	-	278	-	-	
ŭ															
Approach	EB				WB				NB			SB			
HCM Control Delay, s	3.3				0.1				19.9			50.4			
HCM LOS									С			F			
Minor Lane/Major Mvmt	N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1	SBLn2					
Capacity (veh/h)		268	820	-	-	616	-	-	73	635					
HCM Lane V/C Ratio		0.097		-	-	0.007	-	-		0.144					
HCM Control Delay (s)		19.9	10.2	2.3	-	10.9	-	-	122.8	11.6					
		С	В	A		В	-	_	F	В					
HCM Lane LOS			U	, ,						ט					

Intersection						
Int Delay, s/veh	7.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y	LDIN	NDL	4) 	JUIC
Traffic Vol, veh/h	0	82	132	0	3	0
Future Vol, veh/h	0	82	132	0	3	0
Conflicting Peds, #/hr	0	02	0	0	0	0
				Free	Free	Free
Sign Control	Stop	Stop	Free			
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	89	143	0	3	0
Major/Minor I	Minor2	ľ	Major1	١	/lajor2	
Conflicting Flow All	289	3	3	0	-	0
Stage 1	3	-		-	_	-
Stage 2	286	_	_	_	_	_
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	0.22	4.12	_	-	_
	5.42		-	-	-	-
Critical Hdwy Stg 2		-	2 210	-	-	-
Follow-up Hdwy	3.518	3.318		-	-	-
Pot Cap-1 Maneuver	702	1081	1619	-	-	-
Stage 1	1020	-	-	-	-	-
Stage 2	763	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	640	1081	1619	-	-	-
Mov Cap-2 Maneuver	640	-	-	-	-	-
Stage 1	930	-	-	-	-	-
Stage 2	763	-	-	-	-	-
, and the second se						
Annroach	EB		NB		SB	
Approach						
HCM Control Delay, s	8.6		7.4		0	
HCM LOS	A					
Minor Lane/Major Mvm	nt	NBL	NBT I	EBLn1	SBT	SBR
Capacity (veh/h)		1619		1081	_	_
HCM Lane V/C Ratio		0.089		0.082	_	_
HCM Control Delay (s)		7.4	0	8.6	_	_
HCM Lane LOS		Α.4	A	Α	_	_
)	0.3	-	0.3	_	
HCM 95th %tile Q(veh)						

APPENDIX E

Intersection Capacity Analysis Results – Build Conditions (2027)

Verenent EBU EBL EBT EBR WBU WBL WBT WBR NBL NBT NBR SBL SBT SBR NBC Configurations	Intersection															
The Configurations The Configuration The Configura	Int Delay, s/veh	4.5														
Iffic Vol. yehrhh	Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Iffic Vol. yehrhh	Lane Configurations			4Tb			ă	44	1		44		*		1	
Inflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Traffic Vol, veh/h	1	71		4	3				3		2		0		
Control Free Free	Future Vol, veh/h	1	71	621	4	3	5	629	33	3	0	2	65	0	154	
Channelized None None None None None None rage Length 125 - 0 None 75 - 0 10 m Median Storage, # - 0 - 0 125 - 0 0 0 - 0 - 0 - 0 - 0 - 0 -	Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
rage Length	Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
ni Median Storage, # 0 0 0 0 - 0 - 0 - 0	RT Channelized	-	-	-	None	-	-	-	None	-	-	None	-	-	None	
Ade, % 0 0 0 0 0 0 - 0 0 0 0 0 0 0 0 0 0 0 0 - 0 0 -	Storage Length		-	-	-	-	125	-	0	-	-	-	75	-	0	
ak Hour Factor 95 95 95 95 95 95 95 95 95 95 95 95 95	Veh in Median Storage,	# -	-	0	-	-	-	0	-	-	0	-	-	0	-	
avy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	Grade, %															
Major Major Major Major Minor Mino	Peak Hour Factor	95			95	95	95		95							
jor/Minor Major1 Major2 Minor1 Minor2 Inflicting Flow All 662 697 0 0 658 658 0 0 0 1155 1521 329 1157 - 331 Stage 1 808 808 - 678 Stage 2 347 713 - 479 Stage 1 347 713 - 479 Stage 1 808 808 - 678 Stage 2 6.54 6.54 6.94 7.54 - 6.94 Itical Hdwy 644 4.14 6.44 4.14 7.54 6.54 6.54 Stage 1 6.54 5.54 6.54 - 6.54 Idical Hdwy Stg 1 6.54 5.54 6.54 Idical Hdwy Stg 2 6.54 5.54 6.54 - 6.54 Idical Hdwy Stg 2 6.54 5.54 6.54 6.54	Heavy Vehicles, %															
Stage 1	Mvmt Flow	1	75	654	4	3	5	662	35	3	0	2	68	0	162	
Stage 1																_
Stage 1									N							
Stage 2	Conflicting Flow All	662	697	0	0	658	658	0	0			329		-	331	
ficial Hdwy 6.44 4.14 - 6.44 4.14 - 7.54 6.54 6.94 7.54 - 6.94 ficial Hdwy Stg 1	ū	-	-	-	-	-	-	-	-			-		-	-	
icical Hdwy Stg 1		-	-	-	-	-	-	-	-					-	-	
Stage 1	Critical Hdwy	6.44	4.14	-	-	6.44	4.14	-	-			6.94		-	6.94	
Now-up Howy	Critical Hdwy Stg 1	-	-	-	-	-	-	-	-			-		-	-	
Cap-1 Maneuver	Critical Hdwy Stg 2		-	-	-		-	-	-					-	-	
Stage 1 - - - - - 341 392 - 408 0 - Stage 2 - - - - - - 642 434 - 537 0 - toon blocked, % -	Follow-up Hdwy			-	-			-	-							
Stage 2 - - - - - 642 434 - 537 0 - toon blocked, % - <t< td=""><td></td><td>546</td><td>895</td><td>-</td><td>-</td><td>549</td><td>926</td><td>-</td><td>-</td><td></td><td></td><td>667</td><td></td><td></td><td>665</td><td></td></t<>		546	895	-	-	549	926	-	-			667			665	
toon blocked, % v Cap-1 Maneuver 881 881 735 735 102 100 667 134 - 665 v Cap-2 Maneuver 102 100 - 134 Stage 1 295 339 - 353 Stage 2 480 429 - 463		-	-	-	-	-	-	-	-						-	
v Cap-1 Maneuver 881 881 - 735 735 - 102 100 667 134 - 665 v Cap-2 Maneuver - - - - - - 102 100 - 134 - - Stage 1 - - - - - - 295 339 - 353 - - Stage 2 -		-	-	-	-	-	-	-		642	434	-	537	0	-	
V Cap-2 Maneuver - - - - - 102 100 - 134 - - Stage 1 - - - - - 295 339 - 353 - - Stage 2 - - - - - 480 429 - 463 - - Oroach EB WB NB SB SB SB SB SB M Control Delay, s 1.9 0.1 29.2 25.5 D D D D D D D D D D D D D D D D D D D A A - - 735 - - 134 665 665 D A -		001	001	-	-	705	705			100	100	//7	101		//-	
Stage 1 - - - - 295 339 353 - Stage 2 - - - - - - 480 429 - 463 - Oroach EB WB NB SB M Control Delay, s 1.9 0.1 29.2 25.5 M LOS D D D Nor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 SBLn2 Doacity (veh/h) 154 881 - - 735 - - 134 665 M Lane V/C Ratio 0.034 0.085 - - 0.011 - - 57 12.2 M Lane LOS D A A - A - F B			881	-	-	/35	/35	-							665	
Stage 2 - </td <td>•</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>-</td> <td></td>	•	-	-	-	-	-	-	-							-	
NB SB M Control Delay, s 1.9 0.1 29.2 25.5 M LOS D D		-	-	-	-	-	-	-	-					-	-	
M Control Delay, s 1.9 0.1 29.2 25.5 M LOS D D Nor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 SBLn2 Deacity (veh/h) 154 881 735 134 665 M Lane V/C Ratio 0.034 0.085 0.011 0.511 0.244 M Control Delay (s) 29.2 9.5 1 - 10 57 12.2 M Lane LOS D A A - A - F B	Stage 2	-	-	-	-	-	-	-	-	480	429	-	403	-	-	
M Control Delay, s 1.9 0.1 29.2 25.5 M LOS D D Nor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 SBLn2 Deacity (veh/h) 154 881 735 134 665 M Lane V/C Ratio 0.034 0.085 0.011 0.511 0.244 M Control Delay (s) 29.2 9.5 1 - 10 57 12.2 M Lane LOS D A A - A - F B	Annroach	ĘD.				MD				ND			CD			
M LOS D D Nor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 SBLn2 pacity (veh/h) 154 881 735 134 665 M Lane V/C Ratio 0.034 0.085 0.011 0.511 0.244 M Control Delay (s) 29.2 9.5 1 - 10 - 57 12.2 M Lane LOS D A A - A - F B																
Mor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 SBLn2 Deacity (veh/h) 154 881 735 134 665 M Lane V/C Ratio 0.034 0.085 0.011 0.511 0.244 M Control Delay (s) 29.2 9.5 1 - 10 57 12.2 M Lane LOS D A A - A - F B		1.9				U. I										
M Lane V/C Ratio 0.034 0.085 0.011 0.511 0.244 M Control Delay (s) 29.2 9.5 1 - 10 57 12.2 M Lane LOS D A A - A - F B	LICIVI EUS									D			U			
M Lane V/C Ratio 0.034 0.085 0.011 0.511 0.244 M Control Delay (s) 29.2 9.5 1 - 10 57 12.2 M Lane LOS D A A - A - F B	Minor Lano/Major Mumt	N	JRI n1	EDI	EDT	EDD	\\/DI	\\/DT	WPD	SRI n1	SBI n2					
M Lane V/C Ratio 0.034 0.085 0.011 0.511 0.244 M Control Delay (s) 29.2 9.5 1 - 10 57 12.2 M Lane LOS D A A - A - F B		ľ														
M Control Delay (s) 29.2 9.5 1 - 10 57 12.2 M Lane LOS D A A - A - F B								-								
M Lane LOS D A A - A F B								-	-							
					-				-							
1VI 75011 70011E Q(VEIT) 0.1 0.5 0 2.4 1																
	HOW FOUT WITH Q(VEH)		U. I	0.3	-	-	U	-	-	2.4						

Intersection						
Int Delay, s/veh	1.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	N/F		ĵ.			4
Traffic Vol, veh/h	47	0	43	16	0	163
Future Vol, veh/h	47	0	43	16	0	163
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	_	0	_	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	51	0	47	17	0	177
WWW. TOW	01	U		. ,	U	.,,
	Minor1		/lajor1		Major2	
Conflicting Flow All	233	56	0	0	64	0
Stage 1	56	-	-	-	-	-
Stage 2	177	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	755	1011	-	-	1538	-
Stage 1	967	-	-	-	-	-
Stage 2	854	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	755	1011	-	-	1538	-
Mov Cap-2 Maneuver	755	-	-	-	-	-
Stage 1	967	-	-	-	-	-
Stage 2	854	-	-	-	-	-
Annraach	MD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	10.1		0		0	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)			-	755	1538	
HCM Lane V/C Ratio		-	-	0.068	-	-
HCM Control Delay (s)		-	-	10.1	0	-
HCM Lane LOS		-	-	В	Α	-
HCM 95th %tile Q(veh))	-	-	0.2	0	-

Int Delay, s/veh 8.5 Movement EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR Lane Configurations
rane Connonianotis 415 415
Traffic Vol, veh/h 0 0 147 15 0 0 38 0 5 0 1 0
Future Vol, veh/h 0 0 147 15 0 0 38 0 5 0 1 0
Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0
Sign Control Stop Stop Stop Stop Stop Free Free Free Free Free Free
RT Channelized None None None
Storage Length
Veh in Median Storage, # - 0 0 0 -
Grade, % - 0 0 0 -
Peak Hour Factor 92 92 92 92 92 92 92 92 92 92 92 92 92
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2
Mvmt Flow 0 0 160 16 0 0 41 0 5 0 1 0
Major/Minor Minor2 Minor1 Major1 Major2
Conflicting Flow All 86 88 1 166 86 3 1 0 0 5 0 0
Stage 1 1 1 - 85 85
Stage 2 85 87 - 81 1
Critical Hdwy 7.12 6.52 6.22 7.12 6.52 6.22 4.12 - 4.12 -
Critical Hdwy Stg 1 6.12 5.52 - 6.12 5.52
Critical Hdwy Stg 2 6.12 5.52 - 6.12 5.52
Follow-up Hdwy 3.518 4.018 3.318 3.518 4.018 3.318 2.218 2.218 -
Pot Cap-1 Maneuver 900 802 1084 798 804 1081 1622 1616
Stage 1 1022 895 - 923 824
Stage 2 923 823 - 927 895
Platoon blocked, %
Mov Cap-1 Maneuver 883 782 1084 667 784 1081 1622 1616
Mov Cap-2 Maneuver 883 782 - 667 784
3
Stage 2 900 802 - 790 895
Approach EB WB NB SB
HCM Control Delay, s 8.9 10.5 6.4 0
HCM LOS A B
Minor Lane/Major Mvmt NBL NBT NBR EBLn1WBLn1 SBL SBT SBR
Capacity (veh/h) 1622 1084 667 1616
HCM Lane V/C Ratio 0.025 0.147 0.024
HCM Control Delay (s) 7.3 0 - 8.9 10.5 0
HCM Lane LOS A A - A B A
HCM 95th %tile Q(veh) 0.1 0.5 0.1 0

Int Delay, s/veh	11.4														
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations			414			Ä	^	7		4		7		7	
Traffic Vol, veh/h	1	155	801	1	1	3	666	73	3	0	21	55	0	115	
Future Vol, veh/h	1	155	801	1	1	3	666	73	3	0	21	55	0	115	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	-	None	-	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	125	-	0	-	-	-	75	-	0	
Veh in Median Storage,	# -	-	0	-	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	-	0	-	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	1	168	871	1	1	3	724	79	3	0	23	60	0	125	
Major/Minor N	lajor1			ľ	Major2			N	Minor1		N	Minor2			
Conflicting Flow All	724	803	0	0	872	872	0	0	1580	2021	436	1506	-	362	
Stage 1	-	-	-	-	-	-	-	-	1210	1210	-	732	-		
Stage 2	_		_	_	_	_	_	_	370	811	_	774	_	_	
Critical Hdwy	6.44	4.14	_	_	6.44	4.14	_	_	7.54	6.54	6.94	7.54	-	6.94	
Critical Hdwy Stg 1	-		_	_	-		_	_	6.54	5.54	-	6.54	_	-	
Critical Hdwy Stg 2	_	_	_	_	_	-	_	_	6.54	5.54	_	6.54	_	_	
Follow-up Hdwy	2.52	2.22	_	_	2.52	2.22	_	_	3.52	4.02	3.32	3.52	_	3.32	
Pot Cap-1 Maneuver	499	817	_		401	769	_	_	73	57	568	83	0	635	
Stage 1		017	_		- 101	707	_	_	194	254	-	379	0	-	
Stage 2	-	-	-	-	-	-	-		622	391	-	357	0	-	
Platoon blocked, %	_	-	_	-	-	-		-	UZZ	371	-	337	U	-	
Mov Cap-1 Maneuver	812	812	-	-	616	616	-		40	34	568	~ 54	_	635	
				-		010		-	40	34		~ 54		033	
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	-			-		-	-	
Stage 1	-	-	-	-	-	-		-	115	151	-	225	-	-	
Stage 2	-	-	-	-	-	-	-	-	496	389	-	204	-	-	
Angroach	ED				WD				ND			CD			
Approach	EB				WB				NB			SB			
HCM Control Delay, s	4.3				0.1				24.1			98.6			
HCM LOS									С			F			
Minor Long/Main D.C.		NDL 1	ED!	CDT	EDD	MDI	MOT	WDD	2014	CDL C					
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WRK :		SBLn2					
Capacity (veh/h)		214	812	-	-	616	-	-	54	635					
HCM Lane V/C Ratio		0.122		-	-	0.007	-	-		0.197					
HCM Control Delay (s)		24.1	10.6	3.1	-	10.9	-	-	279.6	12.1					
HCM Lane LOS		С	В	Α	-	В	-	-	F	В					
HCM 95th %tile Q(veh)		0.4	0.8	-	-	0	-	-	5.1	0.7					
Notes															

Intersection						
Int Delay, s/veh	1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		\$			4
Traffic Vol, veh/h	31	0	146	44	0	95
Future Vol, veh/h	31	0	146	44	0	95
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	_	-
Veh in Median Storage		_	0	_	_	0
Grade, %	0	-	0	_	_	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	34	0	159	48	0	103
IVIVIIIL I IOW	34	U	137	40	U	103
Major/Minor N	Minor1	<u> </u>	Major1	<u> </u>	Major2	
Conflicting Flow All	286	183	0	0	207	0
Stage 1	183	-	-	-	-	-
Stage 2	103	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	704	859	-	-	1364	-
Stage 1	848	-	-	-	-	-
Stage 2	921	-	-	-	-	-
Platoon blocked, %			-	_		_
Mov Cap-1 Maneuver	704	859	-	_	1364	_
Mov Cap-2 Maneuver	704	-	-	-	-	_
Stage 1	848	-	_	_	_	_
Stage 2	921	_	_	_	_	_
Stuge 2	721					
Approach	WB		NB		SB	
HCM Control Delay, s	10.4		0		0	
HCM LOS	В					
Minor Lane/Major Mvm	n†	NBT	NBRV	WBLn1	SBL	SBT
Capacity (veh/h)	ic .	וטוו	-		1364	JD1 -
HCM Lane V/C Ratio		-		0.048		
HCM Control Delay (s)		-	-		0	-
HCM Lane LOS		-	-	10.4 B	A	-
HCM 95th %tile Q(veh)	١	-	_	0.2	0	
HOW FOUT JOINE CE(VEIT)	J			0.2	U	_

Intersection												
Int Delay, s/veh	7.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	82	10	0	0	132	0	14	0	3	0
Future Vol, veh/h	0	0	82	10	0	0	132	0	14	0	3	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	89	11	0	0	143	0	15	0	3	0
Major/Minor N	Minor2			Minor1			Major1		1	Major2		
Conflicting Flow All	297	304	3	342	297	8	3	0	0	15	0	0
Stage 1	3	3	-	294	294	-	-	-	-	-	-	-
Stage 2	294	301	-	48	3	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	655	609	1081	612	615	1074	1619	-	-	1603	-	-
Stage 1	1020	893	-	714	670	-	-	-	-	-	-	-
Stage 2	714	665	-	965	893	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	610	555	1081	523	560	1074	1619	-	-	1603	-	-
Mov Cap-2 Maneuver	610	555	-	523	560	-	-	-	-	-	-	-
Stage 1	929	893	-	650	610	-	-	-	-	-	-	-
Stage 2	650	606	-	885	893	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	8.6			12			6.7			0		
HCM LOS	Α			В								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1619	-		1081	523	1603	-	_			
HCM Lane V/C Ratio		0.089	_		0.082		-	_	_			
HCM Control Delay (s)		7.4	0	-	8.6	12	0	-	-			
HCM Lane LOS		Α	A	-	A	В	A	-	_			
HCM 95th %tile Q(veh))	0.3	-	-	0.3	0.1	0	-	-			

APPENDIX F

Turn Lane Warrant Assessment

Right Turn Lane Warrant Assessment

Two-Lane Highways

Based on NCHRP Report 279 / VDOT RDM Appendix F
"Intersection Channelization Guide"

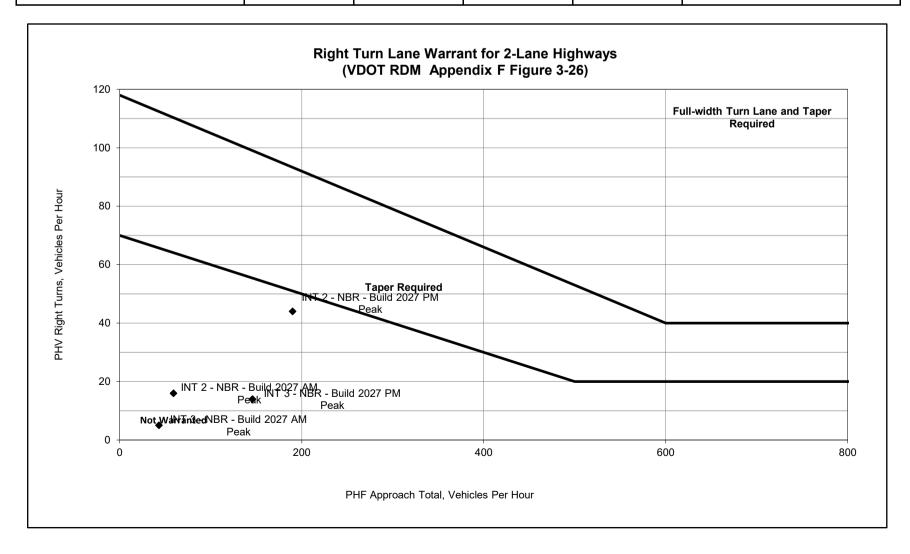
Background:

Warrants for right-turn storage lanes on two-lane highways at unsignalized intersections are based on Figure 3-26 in Appendix F of the Virginia Department of Transportation's (VDOT) *Road Design Manual* (RDM). This figure provides a graphical representation for determining the necessity of a right turn lane and / or taper by comparing the total volumes of a given approach with their respective right turn volumes.

Project Information:

Project:	
Project ID:	
Intersection(s) and Movement(s):	1 -
	2 - Houchins Road at South Site Driveway
	3 - Houchins Road at Crosscreek Drive / North Site Driveway
	4 -
	5 -
Scenario:	Build (2027) Conditions
Analyst:	Gorove Slade

Study Scenario	Approach Volume	Right Turn Volume	Minimum Right Turn Taper Threshold	Minimum Right Turn Full Lane Threshold	Treatment		
INT 2 - NBR - Build 2027 AM Peak	59	16	64	110	Not Warranted		
INT 2 - NBR - Build 2027 PM Peak	190	44	51	93	Not Warranted		
INT 3 - NBR - Build 2027 AM Peak	43	5	66	112	Not Warranted		
INT 3 - NBR - Build 2027 PM Peak	146	14	55	99	Not Warranted		



APPENDIX G

VDOT Pre-Scope Form

THIS IS A NOT CHAPTER 870 STUDY



PRE-SCOPE OF WORK MEETING FORM

Information on the Project Traffic Impact Analysis Base Assumptions

The applicant is responsible for entering the relevant information and submitting the form to VDOT and the locality no less than three (3) business days prior to the meeting. If a form is not received by this deadline, the scope of work meeting may be postponed.

Contact Information										
Consultant Name: Tele: E-mail:	Mike Bailey, P.E. – Gorove Slade Associates, Inc. 804-310-6040 mb@goroveslade.com									
Developer/Owner Name: Tele: E-mail:	John Neel – Foresight Design Services 1260 Radford Street Christiansburg, VA 24073									
Project Information										
Project Name:	Houchins Road Locality/County: Montgomery County									
Project Location: (Attach regional and site specific location map)	See Figure 1									
Submission Type	Comp Plan 🗌	omp Plan ☐ REZ/SUP ⊠ Site Plan ☐ Subd Plat ☐								
Project Description: (Including details on the land use, acreage, phasing, access location, etc. Attach additional sheet if necessary)	The proposed development is planned to occupy two parcels of land, which can be identified on Montgomery County's Parcel Viewer with the Tax Map IDs 080-A46 and 080-A44. The two parcels total approximately 26 acres and are both zoned as M-1 (Industrial Manufacturing) by Montgomery County. The proposed development plan includes the construction of 171 townhomes. The proposed site access includes two full-movement driveways on Houchins Road (Route 758).									
	Residential 🛚	Comme	rcial 🗌	Mix	ed Use	Other 🗌				
Proposed Use(s): (Check all that apply; attach additional pages as necessary)	Other Use(s) ITE LU Code(s): 21 Attached Housing) Square Ft or Other		-	Commercial Use(s) ITE LU Code(s): n/a Square Ft or Other Variable: n/a						
Total Peak Hour Trip Projection:	Less than 100 🗵	100 - 49	99 🗌	500	- 999 🗌	1,000 or more				

Traffic Impact Analysis Assumptions											
Study Period		sting Year: 20		Build-out Year: 2027	7		Design Year: 2027				
Study Area Boundaries	North: See Figure 1					South:					
(Attach map)	West:					East:					
External Factors That Could Affect Project (Planned road improvements, other nearby developments)	ТВІ)									
Consistency With Comprehensive Plan (Land use, transportation plan)	The area		Cou	nty Comprehensive Pla	an list	s this	area as an urban expansion				
Available Traffic Data (Historical, forecasts)	VDO	OT Historical A	AAD'	T Data (2016-2021)							
Trip Distribution		Road Name: Roanoke Street (to/from the East) – 25%					Road Name:				
(Please refer to attached Figure 2 in Supplement)	Road Name: Roanoke Street (to/from the West) - 75%					Road Name:					
Annual Vehicle Trip Growth Rate:	1.0% / yr (che			ak Period for Study eck all that apply)	$\boxtimes A$	⊠ AM ⊠ PM □ SAT					
GI O WEIL TRACE!				ak Hour of the Adj. De used in study)							
	1.	Roanoke Street (U.S. 11/U.S. 460 BUS) at Houchins Road / Bristol Drive									
Study Intersections	2.	Houchins Road at Proposed Site Driveway									
and/or Road Segments (Attach additional sheets as necessary)	3.	Houchins Road at Crosscreek Drive / Proposed Site Driveway			9.						
(Please refer to attached Figure 1.)	4.	4.									
	5.	5.									
	6.										
Trip Adjustment Factors		Internal allowance Reduction:			Pass-by allowance Reduction: ☐ Yes ☐ No						
Software Methodology	Synchro ☐ HCS (v.2000/+) ☐ SIDRA ☐ CORSIM ☐ Other										

Traffic Signal Proposed or Affected (Analysis software to be used, progression speed, cycle length)	Existing traffic signals that could be affected: None Analysis Software: Synchro version 11 Results: HCM 6 Methodology (See Note 6) Queue Lengths to be Reported: 95th Percentile
Improvement(s) Assumed or to be Considered	TBD
Background Traffic Studies Considered	Walnut Creek Residential – 22 single-family detached, 145 townhomes
Plan Submission	
Additional Issues to be Addressed	

NOTES on ASSUMPTIONS:

- 1. Traffic signal timings will be obtained from VDOT, if necessary.
- 2. The scenarios to be included in the study are Existing Conditions (2023), No-Build (2025), and Build (2025). The study will analyze the weekday AM and PM peak hours.
- 3. 2023 existing "baseline" condition counts will be collected at the study intersections.
- 4. Existing peak hour factors will be based on the traffic counts and utilized on a by-intersection basis. Peak hour factors by intersection in the range of 0.85 to 1.00 will be used for existing scenario. Peak hour factors of 0.92 will be used for all future scenarios if the existing peak hour factor by intersection is less than 0.92.
- 5. Heavy vehicle percentages (HV%) will be based on count data.
- 6. HCM 6 methodology will be utilized where applicable; HCM 2000 methodology will be utilized if HCM 6 methodology is not applicable.
- 7. Turn lane warrants will be assessed at the proposed site driveway(s).

Table 1: ITE Trip Generation (11th Edition)

				W e e k d a y						
Land Use	ITE Code	Size Units	AM Peak Hour			PM Peak Hour			Weekday	
	Code		ln	Out	Total	ln	Out	Total	Total	
Single-Family Attached Housing	215	171 d.u.	21	62	83	58	41	99	1,254	

Figure 1: Site Location and Study Intersections



Figure 2: Direction of Approach

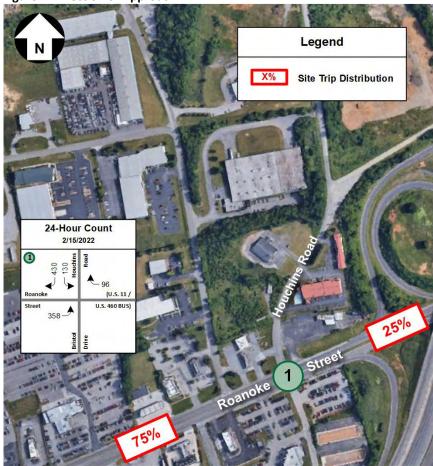


Figure 3: Proposed Site Plan

