## Walnut Ridge

## Rezoning Application

Located in:

## Montgomery County, Virginia

Project Number: 3246.0
Date: March 31, 2023

## Walnut Ridge

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## Walnut Ridge

Rezoning Application

Rezoning Application Form
Rezoning, Conditional Zoning, Proffer Amendment
Montgomery County, Virginia
755 Roanoke St. Suite 2A, Christiansburg, VA 24073
540-394-2148 | mcplan@montgomerycountyva.gov

Applicant Information: (PLEASE PRINT - if additional owners, please attach additional sheets)

| Owner of Record (attach separate page for add'l owners): | Address: |
| :--- | :--- |
| SHAH Development, LLC | P.O. Box 1499, Christiansburg, VA 24068 |
| Telephone: | Email: |
| $(540) 381-8429$ | kconner@shahllc.com |


| Applicant Name: Owner Contract Purchaser/Lessee | Address: |  |
| :--- | :--- | :--- |
| SHAH Development, LLC |  | P.O. Box 1499, Christiansburg, VA 24068 |
| Telephone: | Email: |  |
| $\mathbf{( 5 4 0 )} \mathbf{3 8 1 - 8 4 2 9}$ |  | kconner@shahllc.com |


| Representative Name and Company: <br> Foresight Design Services | Address: <br> 1260 Radford Street, Christiansburg, VA 24073 |
| :--- | :--- |
| Telephone: | Email: |
| (540) 381-6011 | info@foresightdesignservices.com |

## Property Description:

| Location or Address: (Describe in relation to nearest intersection) |  |  |  |  |  |
| :--- | :--- | :--- | :---: | :---: | :---: |
| Property lies east of the intersection of Houchins Road and Crosscreek Drive |  |  |  |  |  |
| Parcel ID Numbers): | Acreage: | Existing Zoning: |  |  |  |
| 018437,018441 | 26.718 | M1 |  |  |  |
| Comprehensive Plan Designation: | Existing Use: |  |  |  |  |
| Urban Expansion Area | Vacant Lot/ Wooded area |  |  |  |  |

Description of Request: (Please provide additional information on attached sheet if necessary)

| Proposed Zoning (Include Acreage ): |
| :--- |
| RM1 |
| Proposed Use: |
| Residential - Townhomes |

I certify that the information supplied on this application and on the attachments provided (maps or other information) is accurate and true to the best of my knowledge. In addition, I hereby grant permission to the agents and employees of Montgomery County and State of Virginia to enter the above property for the purposes of processing and reviewing the above application.

If signing on behalf of a Corporation, Partnership, or LLC, please specify your title with the entity and provide documentation clarifying your authority to sign on behalf of the entity.


## Walnut Ridge

Rezoning Application Justification

## Additional Rezoning Requirements

The applicant for rezoning shall provide a statement of justification to address the following items in the application materials to demonstrate what impact the proposed request will have on the County's resources and how the request complies with Montgomery County's Comprehensive Plan.

## Section 10-54(1)(k)(4), Montgomery County Zoning Ordinance

4. Zoning Map Amendments. If the application is for a reclassification of property to a different zoning district classification on the Zoning Map, the applicant shall address all the following in its statement of justification or plat unless not applicable. The Planning Commission shall give reasonable consideration to the following matters:
a. Whether the proposed zoning district classification is consistent with the Comprehensive Plan.
Response: RM-1 is consistent with the Urban Expansion designation for the Comprehensive Plan. Urban Expansion per the Comprehensive Plan is for the development of residential and non-residential projects.
b. Whether there are any changed or changing conditions in the area affected that make the proposed rezoning appropriate.
Response: With Phase III of Walnut Creek approved and ready to build, the current site will compliment Walnut Creek and provide further expansion with additional residential housing in this area.
c. Whether the range of uses in the proposed zoning district classification are compatible with the uses permitted on other property in the immediate vicinity.
Response: The RM-1 zoning for residential is similar to the adjacent Walnut Creek Phase III residential townhomes.
d. Whether adequate utility, sewer and water, transportation, school and other facilities exist or can be provided to serve the uses that would be permitted on the property if it were rezoned.
Response: All utility, water, sewer, transportation and schools are available for the proposed rezoned property.
e. The effect of the proposed rezoning on the County's ground water supply.

Response: There are no effects to ground water sources.
f. The effect of uses allowed by the proposed rezoning on the structural capacity of the soils.
Response: There is no expected impact on the structural capacity of the soils on site.
g. The impact that the uses that would be permitted if the property were rezoned will have upon the volume of vehicular and pedestrian traffic and traffic safety in the vicinity and whether the proposed rezoning uses sufficient measures to mitigate the impact of through construction traffic on existing neighborhoods and school areas.
Response: A full traffic impact analysis has been performed and is in the appendices of this rezoning. The TIA has been previously submitted to VDOT.
h. Whether a reasonably viable economic use of the subject property exists under the current zoning.
Response: The current zoning does provide for industrial use of the existing site; however, to match the adjacent growth of the area and the increase in housing along Houchins road, the residential aspect of the proposed rezoning compliments the Urban Expansion as well as ties into the existing nearby subdivisions. The existing residential traffic on Houchins Road as well as the topography of the subject property does not economically lend itself to an industrial use.
i. The effect of the proposed rezoning on environmentally sensitive land or natural features, wildlife habitat, vegetation, water quality and air quality.
Response: The proposed subdivision will replace the existing wooded area and will have construction activity that will create noise, air pollutants from construction equipment, and removed trees and vegetation from the area. However, once the subdivision is complete, stormwater bioretention facilities will preserve the water quality, street trees and buffer trees will be planted to restore some of the possible habitat lost as well as help with air quality.
j. Whether the proposed rezoning encourages economic development activities in areas designated by the Comprehensive Plan and provides desirable employment and enlarges the tax base.
Response: The proposed rezoning will provide residential housing per the Urban Expansion area designated in the Comprehensive Plan. This will help to increase the population and taxbase.
k. Whether the proposed rezoning considers the needs of agriculture, industry, and businesses in future growth.
Response: The proposed rezoning seeks to match the trend of residential housing in the expansion area that will provide additional growth for businesses in the area.
I. Whether the proposed rezoning considers the current and future requirements of the community as to land for various purposes as determined by population and economic studies.

Response: Per the Comprehensive Plan, the urban expansion need for the area is residential and nonresidential. The zoning for this area is typically either industrial or residential along the Town of Christiansburg corporate line. Therefore, by providing residential housing the current and future requirements of the urban expansion area are met.
m . Whether the proposed rezoning encourages the conservation of properties and their values and the encouragement of the most appropriate use of land throughout the County.
Response: The proposed rezoning tends to focus on the appropriate use for expansion and growth in this area of Montgomery County per the Comprehensive Plan. Providing additional housing within the urban expansion area will serve to lessen the pressure for development in areas of the County that are more rural in nature and lack services such as water and sewer.
n . Whether the proposed rezoning considers trends of growth or changes, employment, and economic factors, the need for housing, probable future economic and population growth of the county.
Response: This rezoning considers growth of the area and the needs for residential housing to help with population growth in Montgomery County.
o. The effect of the proposed rezoning on the provision of moderate housing by enhancing opportunities for all qualified residents of Montgomery County.
Response: The proposed townhome development will provide housing opportunities for residents of Montgomery County and in particular the added housing capacity serves to provide movement among all different economic categories through $2^{\text {nd }}$ or $3^{\text {rd }}$ order impacts.
p. The effect of the rezoning on natural, scenic, archaeological, or historic features of significant importance.
Response: No known features will be impacted.

## Comprehensive Plan Justification

## Introduction:

Any development within Montgomery County is viewed by the Board of Supervisors, Planning Commission, County Staff, and Citizens through the prism of the comprehensive plan. The following narrative and analysis will address points within the comprehensive plan and discuss how the proposed use aligns with the vision, goals, and objectives of the comprehensive plan. Please note that below are excerpts from the adopted 2025 Comprehensive Plan and one should refer to the Plan for the full text.

The subject property is identified in the Comprehensive Plan as part of the Urban Expansion Area. The text below is from the Comprehensive Plan, with the bold text demonstrating how the proposal meets the guidelines. Additional comprehensive references are included within the narrative.

## Overview:

The project proposes to allow for 26.718 acres of land be developed into residential housing located north of the Town of Christiansburg Industrial Park near the Walnut Creek Subdivision area in Montgomery County. The entirety of the 26.718 acres is vacant and wooded terrain which is located along Houchins Road. The western area is bounded by Houchins Road and Industrial areas of the Town as well as the Walnut Creek Subdivision. To the north is residential properties, the east by woods, and the south is the corporate limits for the Town as well as industrial properties. The subdivision will consist of up to 199 townhomes with recreational open space amenities, bus access, sidewalks, private roadways as well as utilities infrastructure and stormwater management facilities. Stormwater management will be achieved through the use of an onsite detention facility. Stormwater quality will be handled by the dedication of open space on the site, bio retention cells, and nutrient credits.

## Policy Chapters:

## Planning and Land Use

PLU 1.8.3 Urban Expansion Area Land Use:
a. Urban Expansion Areas are the preferred location for new residential and nonresidential development occurring in the unincorporated areas of Montgomery County.
Discussion - This proposed rezoning will meet the preferred location for residential development.

PLU 1.8.5 Urban Expansion Area Facilities and Utilities:
a. Urban Expansion Areas are or will be served by public sewer and water services provided by the County or by the towns and the city, by mutual agreement.
Discussion - The subdivision will be connected to both water and sewer services through coordination efforts with the Town and Christiansburg and the Montgomery County PSA.

## Conclusion:

The Walnut Ridge Subdivision fits within the Comprehensive Plan for Montgomery County's Urban Expansion Areas by providing residential housing with utilities from both the County and Town.

## Narrative

## Section 1: Project Background

## Rezoning Request:

The property owner, SHAH Development, requests to rezone property identified as Tax Parcel Numbers 080-A-44 and 080-A-46 to Multiple Family- Residential (RM-1) for the development of Walnut Ridge. Currently, the property is zoned Industrial Manufacturing ( $\mathrm{M}-1$ ).

The rezoning request will propose up to 199 townhomes that will consist of three-bedroom units. In addition to residential housing, the project includes construction of new private roads, sidewalks, utility infrastructure, stormwater management facilities, and recreational amenities.

The subject property is identified in the Comprehensive Plan as a future land use of Urban Expansion as well as the property is located adjacent to the corporate limits of the Town of Christiansburg. The property rezoning would support the Comprehensive Plan for expansion and provide residential housing to the area.

## Zoning Details:

- Area: Approximately 26.718 acres total (RM-1)
- Address: Houchins Road, Christiansburg, VA
- Parcel ID: 018437 and 018441
- Tax Parcel Numbers: 080-A-44 and 080-A-46
- Magisterial District: Shawsville
- Current Zoning: M-1
- Maximum Density (per zoning): 8.00 Dwellings/Acres (Townhomes)
- Future Land Use: Urban Expansion Area
- Flood Zone: Entire property is located outside of the FEMA 100-year flood zone
- Open Space: +/- 4.00 acres ( $15 \%$ of property, per zoning)


## Property History:

In 2021, the property was purchased by SHAH development, which was owned by the Sawyers Family who own and operate Sawyers Bus Sales.

## Existing Property Conditions:

As can be seen in the following photographs, the existing property is vacant wooded land though zoned $\mathrm{M}-1$, it has not been developed. The majority of the site is wooded and is in close proximity to the Town of Christiansburg corporate limits and nearby residential developments, such as Walnut Creek. The property contains rolling topography that can easily accommodate residential development.

The Town of Christiansburg corporate limits makes up the southern property boundary. Undeveloped agricultural lands adjoin the property to the east. Residential homes and farmland is located to the north. Houchins Road along with industrial and residential development form the western property boundary.


Proposed Entrance Facing South on Houchins.


Proposed Entrance Facing North on Houchins.


Internal wooded areas of site.


Internal timber areas previously cut.


Stream near northeast corner of property.


View of entrance to Walnut Creek Subdivision from northwest end of property.

## Section 2: Walnut Ridge RM-1 Narrative

## Rezoning Narrative

Walnut Ridge consists of residential development that includes single family attached (townhomes), as well as roads, sidewalks, trails, recreation areas, and open space to serve the residents of the development.

The requested RM-1 rezoning will keep to the Comprehensive Plan for Urban Expansion and help Montgomery County achieve its long-term goals related to land development and housing. This RM-1 rezoning includes a Concept Plan that depicts the location, quantity, and layout of the various elements to be included in Walnut Ridge.

Voluntary proffers are included in this rezoning request to mitigate any impacts that are directly related to the development. The proffers are included in the appendices.

Single-family attached dwellings (townhomes) are designed to appeal to buyers who want a smaller home, smaller lot, and desire to have minimal exterior maintenance responsibilities such as yard upkeep, roof repair, etc. This is also appealing to homeowners who want to be close to the Town limits for access to commercial businesses, a variety of restaurants and major roadways for commutes.

In addition to creating a quality neighborhood, the RM-1 zoning district allows The Walnut Ridge development to be designed and built to achieve the goals identified in various County plans, as is discussed in a later section of this narrative. The proposed development provides numerous benefits to the County, including greater tax revenue, additional housing options for citizens, and residential development near the eastern end of Christiansburg.

## Transportation

Walnut Ridge is in close proximity to Christiansburg, U.S. Route I-81, and U.S. Route 460, which makes commuting to Virginia Tech (the area's largest employer) and other parts of Montgomery County very easy. This location is ideal for many families in the New River Valley that commute for work, events, and shopping.

A Traffic Impact Study was completed for the property in 2023 and is included as part of this rezoning in the appendicies.

The development will provide an adequate bus stop for the new subdivision, coordination with VDOT and Montgomery County Public Schools. This bus stop will be located along Houchins Road and shown on the Conceptual plan.

A location for cluster mailboxes is shown on the Conceptual Plan to facilitate mail delivery and provide adequate room for access for residents. Additional parking was added to this area to allow for vehicle access to the mailboxes.

## Utilities

Public water and sewer are available to serve the property and there is capacity and connections available to serve the development. Each residential lot will be connected to the public water and sewer system. The water and sewer systems will be designed and built to Town of Christiansburg and Montgomery County PSA Sewer standards. The property owner will be responsible for all costs associated with the design and construction of the water and sewer infrastructure.

An availability letter from the PSA is included in the appendices of this rezoning application. The proposed development is within the Town of Christiansburg Water Service Agreement Area.

## Emergency Services

Walnut Ridge property is approximately 3.6 miles ( $6-10$ minutes) from Christiansburg Fire Department. Fire hydrants will be installed throughout the development, at locations approved by Montgomery County, to ensure adequate fire protection is available for residents. There are no anticipated negative impacts to emergency services by developing the property. Two entrances to the development are provided as well as interconnectivity of streets providing multiple routes to access any residence within the community.

## Schools

Walnut Ridge residents would attend Montgomery County Public schools.

Walnut Ridge will work closely with Montgomery County Public Schools to coordinate the location for a school bus stop. A proposed stop with shelter is shown on the master plan.

## Section 3: Compliance with Adopted County Plans

## Comprehensive Plan:

The property is designated as Urban Expansion on the Future Land Use Map, which allows for a variety of commercial and residential uses. Walnut Ridge will contain townhomes which is in keeping with the Future Land Use designation for the property.

Walnut Ridge is consistent with the Montgomery County Comprehensive Plan and will help the County move towards achieving goals identified in the Plan.

The Comprehensive Plan identifies future population projections for the County and the need for new housing units in the unincorporated portions of the County. Urban Expansion Areas are the preferred location for new residential and nonresidential development to occur near town boundaries. These areas are set up to accommodate a full range of residential types which include Townhomes.

Walnut Ridge focuses growth in close proximity to the Town of Christiansburg, which helps control growth outside of areas that are easily served by public services. The property is also
near U.S. Route I-81 and 460. The development can easily be served by water, sewer, and public road access, which limits the costs to the County to accommodate projected growth.

Walnut Ridge meets the goals of the Urban Expansion Area and policies identified in the Comprehensive Plan for Urban Expansion Areas (PLU 1.8). Each one of these items is directly addressed in Walnut Ridge Rezoning, as follows:

- Creates high quality residential design to be compatible with existing neighborhoods.
- Provides a residential expansion to the area that was previously approved by the Board of Supervisors as the Walnut Creek Subdivision.
- Will be served by public water and public sewer.

Walnut Ridge addresses the goals identified in the Housing Section of the Comprehensive Plan (HSG 1.1 and 1.3). One of the primary issues identified in the Comprehensive Plan with regards to housing is the "provision of safe and livable neighborhoods and communities." Walnut Ridge creates a compact, livable, pedestrian oriented housing development with open space and sidewalks to encourage residents to be outside and connected to the goings on of the community.

Walnut Ridge meets the goals of the Transportation Section of the Comprehensive Plan (TRN 1.3 and 1.4). The internal roadway network does not include any cul-de-sacs, has interconnected streets and contains pedestrian oriented facilities. The new roads connect at two points along a VDOT roadway, providing for safe and efficient flow of traffic.

Walnut Ridge rezoning is designed to meet the goals and objectives identified in the Comprehensive Plan.

## Section 4: Walnut Ridge Concept Plan Details

## Concept Plan

Walnut Ridge Concept Plan is included as an exhibit in the appendices. Specific elements of the Concept Plan are further described in this section. Once approved by the Montgomery County Board of Supervisors, Walnut Ridge will be developed in general conformance with the Concept Plan and this narrative.

## Land Uses and Land Development Standards

Walnut Ridge will include Townhome lots that will conform to the RM-1 Zoning District Standards.

## Dwelling Units and Density

The maximum total number of dwelling units for the entire RM-1 shall be 199. The 26.718-acre property will have up to an overall maximum density of 7.45 units per acre. This is lower than
the maximum density of 8.0 units per acre allowed in the Montgomery County Zoning Ordinance for RM-1.

## Adjacent Properties

The Concept Plan has been designed to minimize negative impacts to the adjacent property owners and to allow the development to blend into the existing development patterns. This rezoning request and Concept Plan minimizes potential negative impacts to adjacent property owners, as the property owner wants to create a high-quality development that can be integrated into the fabric of the community.

## Trash Collection

Each townhome will have trash cans for private weekly roadside collection.

## Parking

Overall, there will be a minimum of 3 parking spaces per townhome.

## Internal Transportation Network

Walnut Ridge shall be served by all private roads once they are completed. The minor roads in the RM-1 district shall be private and privately maintained. Each lot shall have direct road access, as is depicted on the Concept Plan.

There will be two roadway connections along Houchins Road.

## Open Space

Open space for the project will include sidewalk, landscaped areas, stormwater management facilities, bio retention facilities, active open space, and other undeveloped property.

As is depicted on the Concept Plan, the main focus of open space for this development is the large central green space central to the townhomes. Approximately 6 acres of open space for the common area is set aside in the north and east corner of the site. This area encircles the stormwater management facilities and provides areas for a pedestrian trail through the area. There is also one (1) 10,000 sqft active recreational area for active recreational use adjacent to the biorentention areas. This will provide the community with a playground area for children, grills for cookouts and other amenities such as the potential for a basketball court or other active recreational amenity. The trail will connect the active recreational area with the rest of the open space and the sidewalks throughout the townhome subdivision. The sidewalk system within the development also provides direct access to another central green space of about 2.5 acres.

Buffers will be provided to meet the zoning requirements along the property lines of the subdivision. A type 4 side and rear buffer will be utilized for the areas along the corporate town limits that are adjacent to more industrial properties in the south. A type 2 side and rear buffer will be utilized for properties in the A-1 agricultural areas to the east and north of the site.

## Stormwater Management

The development includes a robust stormwater management system to adequately capture, treat, and convey stormwater runoff on the property. The system will consist of a combination of a single large detention pond, several bioretention cells to maintain water quality, nutrient credit purchases, and the remaining areas to be permanent open space.

In order to protect the downstream site conditions, the detention pond and bioretention basin cells will capture the increased volume of stormwater and reduce the peak runoff to the streams. This will allow the stormwater to be discharged in a manner that does not drastically change the natural water characteristics or cause erosion. Reduction of storm events will be used to maintain acceptable flow volumes to minimize soil erosion. Pollutants will be removed from the stormwater runoff through various bioretention measures and by utilizing the open space to ensure a high level of water quality standards are met. After construction, a combination of BMP quality measures, such as bioretention areas, will be used to reduce phosphorous loading. Nutrient credits will be purchased; however, no more than $25 \%$ of the total nutrient load reduction requirements would be purchased.

On-site stormwater management will meet all applicable Montgomery County regulations, as well as Virginia Department of Environmental Quality regulations. This system will also be maintained by the Home Owners Association after construction is completed.

## Walnut Ridge

ApPendices

## Walnut Ridge

Appendix A: Voluntary Proffers

# Walnut Ridge PROFFER STATEMENT 

## March 31, 2023

Proffer Statement for the Rezoning Application for Tax Parcels 080-A 44 and 080-A-46 (the "Property") from Industrial Manufacturing ( $\mathrm{M}-1$ ) to Multiple-Family Residential (RM-1).
Pursuant to Section 10-54(1) of the Montgomery County Zoning Ordinance, the Owner hereby voluntarily proffers that the Property which is the subject of this Rezoning Application will be developed in accordance with the following conditions, if and only if, approval is granted, and the property is rezoned as requested. This Proffer Statement shall replace in its entirety any Proffer Statement previously approved for the Property. The Applicant, the Owners, their Successors and Assigns, voluntary proffer the following conditions for the Property as follows:

1. Conceptual Layout

The Property shall be developed in general conformance with the Concept Plan prepared by Foresight Design Services, dated March 31, 2023 (the "Concept Plan").
2. Density

A maximum density of no more than 7.45 units per acre shall be permitted for the entire RM-1 for Townhomes.
3. Maximum Dwelling Units

A maximum of 199 single family attached (townhouses) shall be constructed.
4. Utilities

The Property shall be served by Town of Christiansburg Public Water and Montgomery County Public Service Authority sanitary sewer.
5. Property Management

A property management company and/or homeowner's association shall maintain all community owned grounds, including but not limited to landscaped areas, recreational areas, parking and paved areas, sidewalks, and stormwater management areas.
6. Bus Shelter

Bus shelter shall be a minimum of $5^{\prime} \mathrm{X} 10^{\prime}$ in size, constructed of durable, architecturally sound materials that will withstand continual exposure to the elements along Houchins Road. Specific locations, style, and size to be determined in conjunction with the final site plan and coordinated with Montgomery County Public School staff. Coordination with VDOT will be required to provide suitable bus access to the shelter.
-Signature Page to Follow-

I (we) hereby proffer that the development of the subject property of this application shall be in strict accordance with the conditions set forth in this submission.

SHAH Development, LLC
$\mathrm{By}:$
Name: David Hagan
its:

## Commonwealth of Virginia County of Montgomery

The foregoing instrument was acknowledged before me this $\qquad$ day of $\qquad$

Notary Public (Seal)
My commission expires $\qquad$

## Walnut Ridge

Appendix B: Existing Conditions


## Walnut Ridge

Appendix C: Concept Plan


## Walnut Ridge

## Appendix D: Utilities and SWM Plan



## Walnut Ridge

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Walnut Ridge
Appendix F: Building Elevations



UPPER LEVEL



LOWER LEVEL



OPT. 4' EXTENSION


OPT. OWNER'S BATH W/


UPPER LEVEL


OPT. 4' EXTENSION


OPT. 4' EXTENSION


LOWER LEVEL

## Walnut Ridge

Appendix G: PSA Sewer Availability Letter

March 21, 2023

John Neel<br>Gay and Neel, Inc<br>1260 Radford Street<br>Christiansburg, Virginia 24073

RE: Availability Letter<br>Residential Subdivision<br>Houchens Road<br>Parcel ID $018441,018437,014196$<br>Sewer

Dear Mr. Neel:
Public sanitary sewer can be made available to this proposed residential development along Houchens Road, Parcel ID 018441,018437,014196.

Sanitary sewer service may be provided by a public sewer extension to a point adjacent to all units of this proposed development from the public sewer main located in the Walnut Creek Subdivision. You must verify that there is a minimum of two feet of fall from the building service elevation to the top of the sewer main. If adequate vertical separation does not exist, you will be required to install a public sewer pump station and force main per PSA standards. The owner would be required to obtain public easements for all portions of the sewer line extension in private property. The sewer facility fee is $\$ 3,000.00$ per each residential unit. There is also a $\$ 750.00$ pump station fee for each unit.

The sewer facilities must be designed to PSA standards by an engineer and approved by the PSA prior to construction. The owner would be required to obtain public easements for all portions of the sewer line extension in private property. The owner would be responsible for the cost of the sewer line extensions, public easements, highway permits, and any other associated requirements. These designs should be incorporated into the site development plans for this development and submitted to the PSA for review. Payment of twenty-five percent of the water sewer facility fees for all units of the development would be required prior to approval of the site plan.

Please be advised that all PSA sewer systems have a fixed number of available connections. Connections are reserved by payment of facility and connection fees, provided service is currently available to the subject property.

Also, be advised that this development must also meet all Montgomery County Planning and Zoning Department requirements. The availability of sanitary sewer facilities does not by itself authorize the development of this property.

This letter and stated fees are only valid to April 1, 2024.
If you have questions or need additional clarification on the above information, please contact me at 381-1997.

Sincerely,


Charles E. Campbell
PSA Director
cc: Montgomery Co. Planning Dept.

## Walnut Ridge

ApPendix H: Traffic Impact Analysis

## Traffic Impact Analysis

# Houchins Road Townhomes 

Montgomery County, Virginia

## March 2023

Prepared for:
Foresight Design Services
1260 Radford Street
Christiansburg, VA 24073

## Prepared by:

## GOROVE SLADE <br> Transportation Planners and Engineers

| 4114 Legato Road | 225 Reinekers Lane | 1140 Connecticut Ave NW | 4951 Lake Brook Drive |
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## TRAFFIC IMPACT ANALYSIS

| To: | Jesse Miller, P.E. | VDOT |
| :---: | :--- | :--- |
| Cc: | Brea Hopkins | Montgomery County |
|  | John Neel, P.E. | Foresight Design Services |
| From: | Michael Bailey, P.E., RSP 1 |  |
|  |  |  |
| Date: | March 2023 |  |

## Introduction



Shah Development is proposing to construct a townhome neighborhood with 171 single-family attached homes on the east side of Houchins Road. The proposed access plan includes two full-movement driveways on Houchins Road. The site is expected to be built-out by 2027 .

This Traffic Impact Analysis (TIA) was developed in accordance with Virginia Department of Transportation (VDOT), Montgomery County TIA guidelines, and our TIA scope meeting. A copy of the TIA scoping document is included in the Appendix.

## Scope of the Analysis

The objective of this analysis is to identify potential impacts to the transportation network due to the proposed convenience store. Based on the TIA scoping meeting, the following scenarios were analyzed:

- Existing (2023) Conditions
- No-Build (2027) Conditions
- Build (2027) Conditions

The weekday AM and PM peak hours were studied for the following intersections:

- Roanoke Street (U.S. 11 / U.S. 460 BUS) at Houchins Road / Bristol Drive
- Houchins Road at South Site Driveway
- Houchins Road at Crosscreek Drive / North Site Driveway

Figure 1 shows the site location and study intersections and the site plan is shown in Figure 2.


Figure 1: Site Location and Study Intersections


Figure 2: Preliminary Site Plan (Prepared by Gress Engineering for informational purposes only)

## Existing Conditions

## Existing Roadway Network

A description of the major roadways within the study area is shown in Table 1 and the existing lane configuration is shown in Figure 3.

Table 1: Existing Roadway Network

| Roadway | RTE \# | VDOT Classification | Legal/Design Speed <br> Limit (mph) | AADT* <br> $(\mathrm{vpd})$ |
| :--- | :---: | :---: | :---: | :---: |
| Roanoke Street | U.S. 11/ U.S. 460 BUS | Principal Arterial | 35 mph | 15,000 |
| Houchins Road | 758 | Local Roadway | 25 mph | 860 |
| Bristol Drive | $\mathrm{n} / \mathrm{a}$ | Local Roadway | 25 mph | $\mathrm{n} / \mathrm{a}$ |
| Crosscreek Drive | $\mathrm{n} / \mathrm{a}$ | Local Roadway | 25 mph | 760 |
| V VDOT 2021 ADT Traffic Data |  |  |  |  |



Figure 3: Existing Lane Configuration

## Existing (2023) Traffic Volumes

The weekday AM peak hour (7:00 to 9:00 AM) and PM (4:00 to 6:00 PM) turning movement counts were conducted by Burns Services Inc. during the week of November $28^{\text {th }}$ at the following intersections:

- Roanoke Street (U.S. 11 / U.S. 460 BUS) at Houchins Road / Bristol Drive
- Houchins Road at Crosscreek Drive / North Site Driveway

Through movements were increased to balance traffic volumes between intersections. The peak hour traffic volumes are shown in Figure 4 and the count data is included in the Appendix.


Figure 4: Existing (2023) Peak Hour Traffic Volumes

## No-Build Conditions

## Background Improvements

Based on the scoping meeting, no background improvements were included in this analysis.

## Background Developments

Based on the scoping meeting, one approved development was identified within the vicinity of the site to be included in this analysis. The existing Walnut Creek neighborhood, located along Crosscreek Drive, is approved for the development of an additional 22 single-family detached homes and 145 single-family attached units. The anticipated development traffic is included under No-Build (2027) conditions, and the trip generation is shown in Table 2. Figure 5 shows the anticipated site trip volumes, which were based on the following regional distribution:

- $75 \%$ to / from the west on Roanoke Street
- $25 \%$ to / from the east on Roanoke Street

Table 2: ITE Trip Generation - Walnut Creek Residential - Typical Weekday - $11^{\text {th }}$ Edition

| Land Use | ITE <br> Code | Size Units | AM Peak Hour |  |  | Weekday PM Peak Hour |  |  | Weekday Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | In | Out | Total | In | Out | Total |  |
| Single-Family Detached Housing | 210 | 22 d.u. | 5 | 14 | 19 | 15 | 9 | 24 | 252 |
| Single-Family Attached Housing | 215 | 145 d.u. | 17 | 53 | 70 | 49 | 34 | 83 | 1,054 |
| Total Site Trips |  |  | 22 | 67 | 89 | 64 | 43 | 107 | 1,306 |



Figure 5: WaInut Creek Site Trips

## Regional Traffic Growth

Future traffic volumes were estimated by increasing the existing traffic volumes to the build-out year using an annual growth rate and adding the trips generated by the background development. The no-build peak hour traffic volumes were estimated by applying an annual background growth rate of $1.0 \%$ for four years. Figure 6 shows the No-Build (2027) peak hour traffic volumes.


Figure 6: No-Build (2027) Peak Hour Traffic Volumes

## Build Conditions

## Site Generated Trips

Table 3 shows the trip potential of the proposed store based on the $11^{\text {th }}$ Edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual.

Table 3: ITE Trip Generation - Houchins Road Townhomes - Typical Weekday - 11 ${ }^{\text {th }}$ Edition

| Land Use | ITE Code | Size Units | AM Peak Hour |  |  | Weekday $\qquad$ PM Peak Hour |  |  | Weekday Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | In | Out | Total | In | Out | Total |  |
| Single-Family Attached Housing | 215 | 171 d.u. | 21 | 62 | 83 | 58 | 41 | 99 | 1,254 |

## Primary Site Trip Distribution

The site trip distribution was based on existing traffic patterns. The following regional distribution was applied to the site trips:

- $75 \%$ to / from the west on Roanoke Street
- $25 \%$ to / from the east on Roanoke Street

Figure 7 shows the regional site trip distribution and Figure 8 site trip assignment.


Figure 7: Primary Site Trip Distribution


Figure 8: Primary Site Trip Assignment

## Build (2027) Traffic Volumes

The build traffic volumes were estimated by adding the no-build traffic volumes (Figure 6) and the site trip assignment (Figure 8). Figure 9 shows the Build (2027) peak hour traffic volumes.


Figure 9: Build (2027) Peak Hour Traffic Volumes

## Turn Lane Warrant Analysis

The need for turn lanes was evaluated at the site driveways under Build (2027) conditions. The results of the turn lane warrant analysis are summarized in Table 4.

Table 4: Right-Turn Lane Warrant Analysis (2-Lane)

| Study Scenario | Approach <br> Volume | Right Turn <br> Volume | Minimum Right <br> Turn Taper <br> Threshold | Minimum Right <br> Turn Full Lane <br> Threshold | Treatment |
| :--- | :---: | :---: | :---: | :---: | :---: |

Based on the turn lane warrant analysis, no turn lanes are warranted at the proposed site driveways.

## Capacity Analysis

Capacity analysis was performed at the study intersections during the weekday AM and PM peak hours under all analysis scenarios. Synchro, Version 11 was used to analyze the study intersections based on the Highway Capacity Manual (HCM methodology and includes level of service (LOS), delay, and queue lengths for the turning movements analyzed. SimTraffic queues were based on the maximum of an average of 10 microsimulation runs. The queueing analysis results are summarized in the tables below and the Synchro / SimTraffic output reports are included in the appendix. A future peak hour factor (PHF) of 0.92 was used only if the existing PHF was less than 0.92 , otherwise the existing PHF was used.

For unsignalized intersections, the average delays for the minor street movements are described as short delays (less than 25 seconds), moderate delays (between 25 and 50 seconds), and long delays (greater than 50 seconds). It is common for side street movements to experience long delays during the peak hours at intersections with major thoroughfares.

Table 5 shows the LOS, average delays, and queue lengths for the signalized intersection of Roanoke Street (U.S. 11 / U.S. 460 BUS) at Houchins Road / Bristol Drive.

Table 5: Level-of-Service Summary for Roanoke Street at Houchins Road / Bristol Drive

| Scenario | Intersection (Movement) | Effective Storage Length (ft.) [1] | AM Peak Hour |  |  |  | PM Peak Hour |  |  | Ave. Max Queue (ft.) [2] |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | LOS | Delay (sec/veh) | $\begin{gathered} \text { 95th \% } \\ \text { Queue (ft.) } \end{gathered}$ | Ave. Max Queue (ft.) [2] | LOS | Delay (sec/veh) | $\begin{gathered} \text { 95th \% } \\ \text { Queue (ft.) } \end{gathered}$ |  |
|  |  |  |  | Synchro |  | SimTraffic |  | Synchro |  | SimTraffic |
| Existing (2023) <br> Conditions | Roanoke Street (U.S. 11) [E/W] at Houchins Road / Bristol Drive [N/S] <br> Overall Intersection (TWSC) |  |  |  |  |  |  |  |  |  |
|  | Eastbound Approach |  |  |  |  |  |  |  |  |  |
|  | Eastbound U/Left Eastbound Thru/Right |  | A | 9.2 | 3 | 88 | A | 9.8 | 8 | 96 |
|  | Westbound Approach |  |  |  |  |  |  |  |  |  |
|  | Westbound U/Left Westbound Thru Westbound Right | 125 | A | 9.8 | 0 | 28 | B | 10.9 | 0 | 23 |
|  | Northbound Approach |  | C | 21.2 |  |  | C | 17.3 |  |  |
|  | Northbound Left/Thru/Right |  | C | 21.2 | 3 | 32 | C | 17.3 | 8 | 41 |
|  | Southbound Approach |  | C | 18.2 |  |  | D | 32.1 |  |  |
|  | Southbound Left | 75 | D | 31.0 | 18 | 58 | F | 63.8 | 40 | 54 |
|  | Southbound Right |  | B | 10.8 | 8 | 71 | B | 11.2 | 8 | 47 |
| No-Build (2027) <br> Conditions | Roanoke Street (U.S. 11) [ $E / W]$ at Houchins Road / Bristol Drive [N/S] <br> Overall Intersection (TWSC) |  |  |  |  |  |  |  |  |  |
|  | Eastbound Approach |  |  |  |  |  |  |  |  |  |
|  | Eastbound U/Left Eastbound Thru/Right |  | A | 9.4 | 5 | 119 | B | 10.2 | 13 | 118 |
|  |  |  |  |  |  |  |  |  |  |  |
|  | Westbound U/Left Westbound Thru Westbound Right | 125 | A | 10 | 0 | 24 | B | 10.9 | 0 | 27 |
|  | Northbound Approach |  | D | 25.5 |  |  | C | 19.9 |  |  |
|  | Northbound Left/Thru/Right |  | D | 25.5 | 3 | 2 | C | 19.9 | 8 | 41 |
|  | Southbound Approach |  | C | 21.6 |  |  | $F$ | 50.4 |  |  |
|  | Southbound Left | 75 | E | 43.3 | 38 | 30 | F | 122.8 | 75 | 70 |
|  | Southbound Right |  | B | 11.5 | 15 | 63 | B | 11.6 | 13 | 128 |
| Build (2027) <br> Conditions | Roanoke Street (U.S. 11) [ $E / W]$ at Houchins Road / Bristol Drive [N/S] Overall Intersection (TWSC) |  |  |  |  |  |  |  |  |  |
|  | Eastbound Approach |  |  |  |  |  |  |  |  |  |
|  | Eastbound U/Left Eastbound Thru/Right |  | A | 9.5 | 8 | 152 | B | 10.6 | 20 | 128 |
|  | Westbound Approach |  |  |  |  |  |  |  |  |  |
|  | Westbound U/Left Westbound Thru Westbound Right | 125 | A | 10 | 0 | 25 | B | 10.9 | 0 | 20 |
|  | Northbound Approach |  | D | 29.2 |  |  | C | 24.1 |  |  |
|  | Northbound Left/Thru/Right |  | D | 29.2 | 3 | 34 | C | 24.1 | 10 | 2 |
|  | Southbound Approach |  | D | 25.5 |  |  | $F$ | 98.6 |  |  |
|  | Southbound Left Southbound Right | 75 | $\begin{aligned} & \mathrm{F} \\ & \mathrm{~B} \end{aligned}$ | $\begin{gathered} 57 \\ 12.2 \end{gathered}$ | $\begin{aligned} & 60 \\ & 25 \end{aligned}$ | $\begin{gathered} 74 \\ 172 \end{gathered}$ | $\begin{aligned} & \mathrm{F} \\ & \mathrm{~B} \end{aligned}$ | $\begin{gathered} 279.6 \\ 12.1 \end{gathered}$ | $\begin{gathered} 128 \\ 18 \end{gathered}$ | $\begin{gathered} 7 \\ 47 \end{gathered}$ |

Capacity analysis shows that the minor street left-turn movement currently operates with moderate delays during the AM peak hour and with long delays during the PM peak hour. Under no-build conditions, the minor street left-turn movement is expected to continue operating with moderate delays during the AM peak hour and with long delays during the PM peak hour. Under build conditions, the minor street left-turn movement is expected to operate with long delays during the AM and PM peak hours. It is common for side street movements and left turns to experience long delays during the peak hours at intersections with major thoroughfares. No improvements are warranted or recommended at this intersection upon build-out of the proposed site.

Table 6 shows the LOS, average delays, and queue lengths for the unsignalized intersection of Houchins Road at the proposed South Site Driveway.

Table 6: Level-of-Service Summary for Houchins Road at South Site Driveway

| Scenario | Intersection (Movement) | AM Peak Hour |  |  |  | PM Peak Hour |  |  | Ave. Max Queue (ft.) [2] |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | LOS | $\begin{gathered} \text { Delay } \\ \text { (sec/veh) } \end{gathered}$ | $\begin{gathered} \text { 95th \% } \\ \text { Queue (ft.) } \end{gathered}$ | Ave. Max Queue (ft.) [2] | LOS | $\begin{gathered} \text { Delay } \\ \text { (sec/veh) } \end{gathered}$ | $\begin{gathered} \text { 95th \% } \\ \text { Queue (ft.) } \end{gathered}$ |  |
|  |  | Synchro |  |  | SimTraffic | Synchro |  |  | SimTraffic |
| $\begin{aligned} & \text { Build } \\ & \text { (2027) } \end{aligned}$ <br> Conditions | Houchins Road [N/S] at South <br> Site Driveway [E/W] <br> Overall Intersection (TWSC) |  |  |  |  |  |  |  |  |
|  | Westbound Approach | B | 10.1 |  |  | B | 10.4 |  |  |
|  | Westbound Left/Right | B | 10.1 | 5 | 56 | B | 10.4 | 5 | 57 |
|  | Northbound Approach |  |  |  |  |  |  |  |  |
|  | Northbound Thru/Right |  |  |  |  |  |  |  |  |
|  | Southbound Approach |  |  |  |  |  |  |  |  |
|  | Southbound Left/Thru |  |  |  |  |  |  |  |  |

Capacity analysis shows that under build conditions, the minor street left-turn movement is expected to operate with short delays and queue lengths of three vehicles or less during the AM and PM peak hours. No improvements are warranted or recommended at the build-out of the proposed site.

Table 7 shows the LOS, average delays, and queue lengths for the unsignalized intersection of Houchins Road at Crosscreek Drive / North Site Driveway.

Table 7: Level-of-Service Summary for Houchins Road at Crosscreek Drive / North Site Driveway


Capacity analysis shows that the minor approach currently operates with short delays and queue lengths of two vehicles or less during the AM and PM peak hours. Under both no-build and build conditions, the minor approaches are expected to continue operating with short delays and queue lengths of three vehicles or less during the AM and PM peak hours. No improvements are warranted or recommended at the build-out of the proposed site.

## Summary and Conclusions

Based on the results of the analysis, all intersections are projected to operate with acceptable delay and queueing upon completion of the proposed townhome neighborhood. No improvements are warranted or recommended at build-out of this site.
Figure 10 shows the recommended lane configuration.


Figure 10: Recommended Lane Configuration

## TECHNICAL APPENDIX

# TECHNICAL APPENDIX 

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Turning Movement Count Sheets

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Intersection Capacity Analysis Results - No-Build Conditions (2027)

APPENDIX E
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## APPENDIX G

VDOT Pre-Scope Form

## APPENDIX A

Turning Movement Count Sheets


TRAFFIC DATA COLLECTION
File Name: Christiansburg(Houchins Rd and US-11)
Site Code :
Start Date : 1/1/2023
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|  | Houchins Road Southbound |  |  |  |  |  | US-11 <br> Westbound |  |  |  |  |  | Bristol Drive SE Northbound |  |  |  |  |  | US-11 <br> Eastbound |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | UTrn | Peds | App. Total | Right | Thru | Left | UTrn | Peds | App. Total | Right | Thru | Left | UTrn | Peds | App. Total | Right | Thru | Left | UTrn | Peds | App. Total | Int. Total |
| 07:00 AM | 14 | 0 | 7 | 0 | 0 | 21 | 5 | 134 | 2 | 0 | 0 | 141 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 81 | 3 | 0 | 0 | 85 | 247 |
| 07:15 AM | 16 | 0 | 14 | 0 | 0 | 30 | 7 | 172 | 0 | 0 | 0 | 179 | 1 | 0 | 0 | 0 | 0 | 1 | 1 | 86 | 8 | 0 | 0 | 95 | 305 |
| 07:30 AM | 18 | 0 | 13 | 0 | 0 | 31 | 4 | 183 | 1 | 0 | 0 | 188 | 0 | 0 | 3 | 0 | 0 | 3 | 5 | 84 | 3 | 0 | 0 | 92 | 314 |
| 07:45 AM | 17 | 0 | 6 | 0 | 0 | 23 | 5 | 185 | 5 | 0 | 0 | 195 | 1 | 1 | 2 | 0 | 0 | 4 | 4 | 98 | 9 | 0 | 0 | 111 | 333 |
| Total | 65 | 0 | 40 | 0 | 0 | 105 | 21 | 674 | 8 | 0 | 0 | 703 | 2 | 1 | 5 | 0 | 0 | 8 | 11 | 349 | 23 | 0 | 0 | 383 | 1199 |
| 08:00 AM | 10 | 0 | 11 | 0 | 0 | 21 | 4 | 152 | 0 | 0 | 0 | 156 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 122 | 9 | 1 | 0 | 132 | 309 |
| 08:15 AM | 15 | 0 | 11 | 0 | 0 | 26 | 7 | 148 | 1 | 1 | 0 | 157 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 149 | 12 | 0 | 0 | 161 | 345 |
| 08:30 AM | 13 | 0 | 5 | 0 | 0 | 18 | 6 | 167 | 2 | 1 | 0 | 176 | 2 | 0 | 2 | 0 | 0 | 4 | 2 | 144 | 8 | 0 | 0 | 154 | 352 |
| 08:45 AM | 17 | 0 | 5 | 0 | 0 | 22 | 4 | 137 | 2 | 1 | 0 | 144 | 0 | 0 | 1 | 0 | 0 | 1 | 2 | 182 | 8 | 0 | 0 | 192 | 359 |
| Total | 55 | 0 | 32 | 0 | 0 | 87 | 21 | 604 | 5 | 3 | 0 | 633 | 2 | 0 | 3 | 0 | 1 | 6 | 4 | 597 | 37 | 1 | 0 | 639 | 1365 |
| Grand Total | 120 | 0 | 72 | 0 | 0 | 192 | 42 | 1278 | 13 | 3 | 0 | 1336 | 4 | 1 | 8 | 0 | 1 | 14 | 15 | 946 | 60 | 1 | 0 | 1022 | 2564 |
| Apprch \% | 62.5 | 0 | 37.5 | 0 | 0 |  | 3.1 | 95.7 | 1 | 0.2 | 0 |  | 28.6 | 7.1 | 57.1 | 0 | 7.1 |  | 1.5 | 92.6 | 5.9 | 0.1 | 0 |  |  |
| Total \% | 4.7 | 0 | 2.8 | 0 | 0 | 7.5 | 1.6 | 49.8 | 0.5 | 0.1 | 0 | 52.1 | 0.2 | 0 | 0.3 | 0 | 0 | 0.5 | 0.6 | 36.9 | 2.3 | 0 | 0 | 39.9 |  |
| Cars + | 116 | 0 | 65 | 0 | 0 | 181 | 40 | 1224 | 13 | 3 | 0 | 1280 | 4 | 1 | 8 | 0 | 1 | 14 | 15 | 847 | 57 | 1 | 0 | 920 | 2395 |
| \% Cars + | 96.7 | 0 | 90.3 | 0 | 0 | 94.3 | 95.2 | 95.8 | 100 | 100 | 0 | 95.8 | 100 | 100 | 100 | 0 | 100 | 100 | 100 | 89.5 | 95 | 100 | 0 | 90 | 93.4 |
| Trucks | 4 | 0 | 7 | 0 | 0 | 11 | 2 | 54 | 0 | 0 | 0 | 56 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 99 | 3 | 0 | 0 | 102 | 169 |
| \% Trucks | 3.3 | 0 | 9.7 | 0 | 0 | 5.7 | 4.8 | 4.2 | 0 | 0 | 0 | 4.2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 10.5 | 5 | 0 | 0 | 10 | 6.6 |

TRAFFIC DATA COLLECTION
File Name : Christiansburg(Houchins Rd and US-11)
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|  | Houchins Road Southbound |  |  |  |  |  | US-11 <br> Westbound |  |  |  |  |  | Bristol Drive SE Northbound |  |  |  |  |  | US-11 Eastbound |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | UTr | Peds | Appo Toal | Right | Thru | Left | UTm | Peds | App. Toal | Right | Thru | Left | UTm | Peds | Apor Toal | Right | Thru | Left | UTr | Peds | , | Int Toa |
| Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 08:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 08:00 AM | 10 | 0 | 11 | 0 | 0 | 21 | 4 | 152 | 0 | 0 | 0 | 156 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 122 | 9 | 1 | 0 | 132 | 309 |
| 08:15 AM | 15 | 0 | 11 | 0 | 0 | 26 | 7 | 148 | 1 | 1 | 0 | 157 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 149 | 12 | 0 | 0 | 161 | 345 |
| 08:30 AM | 13 | 0 | 5 | 0 | 0 | 18 | 6 | 167 | 2 | 1 | 0 | 176 | 2 | 0 | 2 | 0 | 0 | 4 | 2 | 144 | 8 | 0 | 0 | 154 | 352 |
| 08:45 AM | 17 | 0 | 5 | 0 | 0 | 22 | 4 | 137 | 2 | 1 | 0 | 144 | 0 | 0 | 1 | 0 | 0 | 1 | 2 | 182 | 8 | 0 | 0 | 192 | 359 |
| Total Volume | 55 | 0 | 32 | 0 | 0 | 87 | 21 | 604 | 5 | 3 | 0 | 633 | 2 | 0 | 3 | 0 | 1 | 6 | 4 | 597 | 37 | 1 | 0 | 639 | 1365 |
| \% App. Total | 63.2 | 0 | 36.8 | 0 | 0 |  | 3.3 | 95.4 | 0.8 | 0.5 | 0 |  | 33.3 | 0 | 50 | 0 | 16.7 |  | 0.6 | 93.4 | 5.8 | 0.2 | 0 |  |  |
| PHF | 809 | . 000 | . 727 | . 000 | . 000 | . 837 | . 750 | . 904 | . 625 | . 750 | . 000 | . 899 | . 250 | . 000 | . 375 | . 000 | . 250 | 375 | . 500 | . 820 | . 771 | . 250 | . 000 | . 832 | . 951 |




TRAFFIC DATA COLLECTION
File Name : Christiansburg(Houchins Rd and US-11)
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|  | Houchins Road Southbound |  |  |  |  |  | US-11 <br> Westbound |  |  |  |  |  | Bristol Drive SE Northbound |  |  |  |  |  | US-11 <br> Eastbound |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | UTrn | Peds | App. Total | Right | Thru | Left | UTrn | Peds | App. Total | Right | Thru | Left | UTrn | Peds | App. Toal | Right | Thru | Left | UTrn | Peds | App. Total | Int. Total |
| 04:00 PM | 9 | 0 | 11 | 0 | 0 | 20 | 11 | 163 | 1 | 0 | 0 | 175 | 1 | 1 | 0 | 0 | 0 | 2 | 0 | 184 | 17 | 1 | 0 | 202 | 399 |
| 04:15 PM | 9 | 0 | 13 | 0 | 0 | 22 | 5 | 154 | 0 | 0 | 0 | 159 | 1 | 0 | 0 | 0 | 0 | 1 | 0 | 188 | 10 | 0 | 0 | 198 | 380 |
| 04:30 PM | 7 | 0 | 10 | 0 | 0 | 17 | 10 | 140 | 1 | 1 | 0 | 152 | 2 | 0 | 1 | 0 | 0 | 3 | 0 | 170 | 11 | 0 | 0 | 181 | 353 |
| 04:45 PM | 9 | 0 | 4 | 0 | 0 | 13 | 10 | 159 | 0 | 0 | 0 | 169 | 5 | 0 | 0 | 0 | 0 | 5 | 0 | 166 | 15 | 0 | 0 | 181 | 368 |
| Total | 34 | 0 | 38 | 0 | 0 | 72 | 36 | 616 | 2 | 1 | 0 | 655 | 9 | 1 | 1 | 0 | 0 | 11 | 0 | 708 | 53 | 1 | 0 | 762 | 1500 |
| 05:00 PM | 13 | 0 | 7 | 0 | 0 | 20 | 13 | 169 | 2 | 0 | 0 | 184 | 6 | 0 | 1 | 0 | 0 | 7 | 0 | 236 | 14 | 0 | 0 | 250 | 461 |
| 05:15 PM | 21 | 0 | 6 | 0 | 0 | 27 | 8 | 154 | 1 | 0 | 0 | 163 | 6 | 0 | 1 | 0 | 0 | 7 | 0 | 195 | 19 | 0 | 1 | 215 | 412 |
| 05:30 PM | 7 | 0 | 16 | 0 | 0 | 23 | 10 | 158 | 0 | 0 | 0 | 168 | 3 | 0 | 1 | 0 | 0 | 4 | 0 | 173 | 13 | 0 | 0 | 186 | 381 |
| 05:45 PM | 14 | 0 | 4 | 0 | 0 | 18 | 8 | 139 | 1 | 1 | 0 | 149 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 147 | 18 | 0 | 0 | 165 | 333 |
| Total | 55 | 0 | 33 | 0 | 0 | 88 | 39 | 620 | 4 | 1 | 0 | 664 | 15 | 0 | 4 | 0 | 0 | 19 | 0 | 751 | 64 | 0 | 1 | 816 | 1587 |
| Grand Total | 89 | 0 | 71 | 0 | 0 | 160 | 75 | 1236 | 6 | 2 | 0 | 1319 | 24 | 1 | 5 | 0 | 0 | 30 | 0 | 1459 | 117 | 1 | 1 | 1578 | 3087 |
| Apprch \% | 55.6 | 0 | 44.4 | 0 | 0 |  | 5.7 | 93.7 | 0.5 | 0.2 | 0 |  | 80 | 3.3 | 16.7 | 0 | 0 |  | 0 | 92.5 | 7.4 | 0.1 | 0.1 |  |  |
| Total \% | 2.9 | 0 | 2.3 | 0 | 0 | 5.2 | 2.4 | 40 | 0.2 | 0.1 | 0 | 42.7 | 0.8 | 0 | 0.2 | 0 | 0 | 1 | 0 | 47.3 | 3.8 | 0 | 0 | 51.1 |  |
| Cars + | 87 | 0 | 70 | 0 | 0 | 157 | 72 | 1217 | 6 | 2 | 0 | 1297 | 24 | 1 | 5 | 0 | 0 | 30 | 0 | 1434 | 116 | 1 | 1 | 1552 | 3036 |
| \% Cars + | 97.8 | 0 | 98.6 | 0 | 0 | 98.1 | 96 | 98.5 | 100 | 100 | 0 | 98.3 | 100 | 100 | 100 | 0 | 0 | 100 | 0 | 98.3 | 99.1 | 100 | 100 | 98.4 | 98.3 |
| Trucks | 2 | 0 | 1 | 0 | 0 | 3 | 3 | 19 | 0 | 0 | 0 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 25 | 1 | 0 | 0 | 26 | 51 |
| \% Trucks | 2.2 | 0 | 1.4 | 0 | 0 | 1.9 | 4 | 1.5 | 0 | 0 | 0 | 1.7 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1.7 | 0.9 | 0 | 0 | 1.6 | 1.7 |

TRAFFIC DATA COLLECTION
File Name: Christiansburg(Houchins Rd and US-11)
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|  | Houchins Road Southbound |  |  |  |  |  | US-11 <br> Westbound |  |  |  |  |  | Bristol Drive SE Northbound |  |  |  |  |  | US-11 Eastbound |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | UTrn | Peds | App. Toal | Right | Thru | Left | UTrn | Peds | App. Total | Right | Thru | Left | UTrn | Peds | App. Total | Right | Thru | Left | UTrn | Peds | App. Tolal | Int. Total |
| Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 04:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 04:45 PM | 9 | 0 | 4 | 0 | 0 | 13 | 10 | 159 | 0 | 0 | 0 | 169 | 5 | 0 | 0 | 0 | 0 | 5 | 0 | 166 | 15 | 0 | 0 | 181 | 368 |
| 05:00 PM | 13 | 0 | 7 | 0 | 0 | 20 | 13 | 169 | 2 | 0 | 0 | 184 | 6 | 0 | 1 | 0 | 0 | 7 | 0 | 236 | 14 | 0 | 0 | 250 | 461 |
| 05:15 PM | 21 | 0 | 6 | 0 | 0 | 27 | 8 | 154 | 1 | 0 | 0 | 163 | 6 | 0 | 1 | 0 | 0 | 7 | 0 | 195 | 19 | 0 | 1 | 215 | 412 |
| 05:30 PM | 7 | 0 | 16 | 0 | 0 | 23 | 10 | 158 | 0 | 0 | 0 | 168 | 3 | 0 | 1 | 0 | 0 | 4 | 0 | 173 | 13 | 0 | 0 | 186 | 381 |
| Total Volume | 50 | 0 | 33 | 0 | 0 | 83 | 41 | 640 | 3 | 0 | 0 | 684 | 20 | 0 | 3 | 0 | 0 | 23 | 0 | 770 | 61 | 0 | 1 | 832 | 1622 |
| \% App. Total | 60.2 | 0 | 39.8 | 0 | 0 |  | 6 | 93.6 | 0.4 | 0 | 0 |  | 87 | 0 | 13 | 0 | 0 |  | 0 | 92.5 | 7.3 | 0 | 0.1 |  |  |
| PHF | . 595 | . 000 | . 516 | . 000 | . 000 | . 769 | . 788 | . 947 | . 375 | . 000 | . 000 | . 929 | . 833 | . 000 | . 750 | . 000 | . 000 | . 821 | . 000 | . 816 | . 803 | . 000 | . 250 | . 832 | . 880 |




TRAFFIC DATA COLLECTION
File Name : Christiansburg(Houchins Rd atnd Crosscreek Dr)
Site Code :
Start Date : 2/22/2023
Page No : 1

|  | Houchins Road Southbound |  |  |  |  | Houchins Road Northbound |  |  |  |  | Crosscreek Drive Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | UTrn | Peds | App. Total | Thru | Left | UTrn | Peds | App. Total | Right | Left | UTrn | Peds | App. Total | Int. Total |
| 07:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 17 | 0 | 0 | 0 | 17 | 18 |
| 07:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 7 | 23 | 0 | 0 | 0 | 23 | 30 |
| 07:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 21 | 0 | 0 | 0 | 21 | 22 |
| 07:45 AM | 0 | 1 | 0 | 0 | 1 | 0 | 6 | 0 | 0 | 6 | 16 | 0 | 0 | 0 | 16 | 23 |
| Total | 0 | 1 | 0 | 0 | 1 | 0 | 15 | 0 | 0 | 15 | 77 | 0 | 0 | 0 | 77 | 93 |
| 08:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 3 | 12 | 0 | 0 | 0 | 12 | 15 |
| 08:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 7 | 19 | 0 | 1 | 0 | 20 | 27 |
| 08:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 4 | 8 | 0 | 0 | 0 | 8 | 12 |
| 08:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 3 | 13 | 0 | 0 | 0 | 13 | 16 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 0 | 0 | 17 | 52 | 0 | 1 | 0 | 53 | 70 |
| Grand Total | 0 | 1 | 0 | 0 | 1 | 0 | 32 | 0 | 0 | 32 | 129 | 0 | 1 | 0 | 130 | 163 |
| Apprch \% | 0 | 100 | 0 | 0 |  | 0 | 100 | 0 | 0 |  | 99.2 | 0 | 0.8 | 0 |  |  |
| Total \% | 0 | 0.6 | 0 | 0 | 0.6 | 0 | 19.6 | 0 | 0 | 19.6 | 79.1 | 0 | 0.6 | 0 | 79.8 |  |
| Cars + | 0 | 1 | 0 | 0 | 1 | 0 | 30 | 0 | 0 | 30 | 126 | 0 | 1 | 0 | 127 | 158 |
| \% Cars + | 0 | 100 | 0 | 0 | 100 | 0 | 93.8 | 0 | 0 | 93.8 | 97.7 | 0 | 100 | 0 | 97.7 | 96.9 |
| Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 2 | 3 | 0 | 0 | 0 | 3 | 5 |
| \% Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 6.2 | 0 | 0 | 6.2 | 2.3 | 0 | 0 | 0 | 2.3 | 3.1 |

TRAFFIC DATA COLLECTION
File Name : Christiansburg(Houchins Rd atnd Crosscreek Dr)
Site Code :
Start Date : 2/22/2023
Page No : 2

|  | Houchins Road Southbound |  |  |  |  | Houchins Road Northbound |  |  |  |  | Crosscreek Drive Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | UTrn | Peds | App. Total | Thru | Left | UTrn | Peds | App. Total | Right | Left | UTrn | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Entire Intersection Begins at 07:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 07:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 17 | 0 | 0 | 0 | 17 | 18 |
| 07:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 7 | 23 | 0 | 0 | 0 | 23 | 30 |
| 07:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 21 | 0 | 0 | 0 | 21 | 22 |
| 07:45 AM | 0 | 1 | 0 | 0 | 1 | 0 | 6 | 0 | 0 | 6 | 16 | 0 | 0 | 0 | 16 | 23 |
| Total Volume | 0 | 1 | 0 | 0 | 1 | 0 | 15 | 0 | 0 | 15 | 77 | 0 | 0 | 0 | 77 | 93 |
| \% App. Total | 0 | 100 | 0 | 0 |  | 0 | 100 | 0 | 0 |  | 100 | 0 | 0 | 0 |  |  |
| PHF | . 000 | . 250 | . 000 | . 000 | . 250 | . 000 | . 536 | . 000 | . 000 | . 536 | . 837 | . 000 | . 000 | . 000 | . 837 | . 775 |




TRAFFIC DATA COLLECTION
File Name : Christiansburg(Houchins Rd atnd Crosscreek Dr)
Site Code :
Start Date : 2/22/2023
Page No : 1

|  | Houchins Road Southbound |  |  |  |  | Houchins Road Northbound |  |  |  |  | Crosscreek Drive Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | UTrn | Peds | App. Total | Thru | Left | UTrn | Peds | App. Total | Right | Left | UTrn | Peds | App. Total | Int. Total |
| 04:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 0 | 15 | 8 | 0 | 0 | 0 | 8 | 23 |
| 04:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 9 | 0 | 0 | 9 | 5 | 0 | 0 | 0 | 5 | 14 |
| 04:30 PM | 0 | 0 | 0 | 0 | 0 | 1 | 12 | 0 | 0 | 13 | 5 | 0 | 0 | 0 | 5 | 18 |
| 04:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 19 | 0 | 0 | 19 | 7 | 0 | 0 | 0 | 7 | 26 |
| Total | 0 | 0 | 0 | 0 | 0 | 1 | 55 | 0 | 0 | 56 | 25 | 0 | 0 | 0 | 25 | 81 |
| 05:00 PM | 0 | 2 | 0 | 0 | 2 | 0 | 18 | 0 | 0 | 18 | 10 | 0 | 0 | 0 | 10 | 30 |
| 05:15 PM | 0 | 1 | 0 | 0 | 1 | 0 | 15 | 0 | 0 | 15 | 10 | 0 | 0 | 1 | 11 | 27 |
| 05:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 0 | 12 | 10 | 0 | 0 | 0 | 10 | 22 |
| 05:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 20 | 0 | 0 | 20 | 7 | 0 | 0 | 3 | 10 | 30 |
| Total | 0 | 3 | 0 | 0 | 3 | 0 | 65 | 0 | 0 | 65 | 37 | 0 | 0 | 4 | 41 | 109 |
| Grand Total | 0 | 3 | 0 | 0 | 3 | 1 | 120 | 0 | 0 | 121 | 62 | 0 | 0 | 4 | 66 | 190 |
| Apprch \% | 0 | 100 | 0 | 0 |  | 0.8 | 99.2 | 0 | 0 |  | 93.9 | 0 | 0 | 6.1 |  |  |
| Total \% | 0 | 1.6 | 0 | 0 | 1.6 | 0.5 | 63.2 | 0 | 0 | 63.7 | 32.6 | 0 | 0 | 2.1 | 34.7 |  |
| Cars + | 0 | 3 | 0 | 0 | 3 | 1 | 118 | 0 | 0 | 119 | 60 | 0 | 0 | 4 | 64 | 186 |
| \% Cars + | 0 | 100 | 0 | 0 | 100 | 100 | 98.3 | 0 | 0 | 98.3 | 96.8 | 0 | 0 | 100 | 97 | 97.9 |
| Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 2 | 2 | 0 | 0 | 0 | 2 | 4 |
| \% Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 1.7 | 0 | 0 | 1.7 | 3.2 | 0 | 0 | 0 | 3 | 2.1 |

TRAFFIC DATA COLLECTION
File Name : Christiansburg(Houchins Rd atnd Crosscreek Dr)
Site Code :
Start Date : 2/22/2023
Page No : 2

|  | Houchins Road Southbound |  |  |  |  | Houchins Road Northbound |  |  |  |  | Crosscreek Drive Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | UTrn | Peds | App. Total | Thru | Left | UTrn | Peds | App. Total | Right | Left | UTrn | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Entire Intersection Begins at 05:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 05:00 PM | 0 | 2 | 0 | 0 | 2 | 0 | 18 | 0 | 0 | 18 | 10 | 0 | 0 | 0 | 10 | 30 |
| 05:15 PM | 0 | 1 | 0 | 0 | 1 | 0 | 15 | 0 | 0 | 15 | 10 | 0 | 0 | 1 | 11 | 27 |
| 05:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 0 | 12 | 10 | 0 | 0 | 0 | 10 | 22 |
| 05:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 20 | 0 | 0 | 20 | 7 | 0 | 0 | 3 | 10 | 30 |
| Total Volume | 0 | 3 | 0 | 0 | 3 | 0 | 65 | 0 | 0 | 65 | 37 | 0 | 0 | 4 | 41 | 109 |
| \% App. Total | 0 | 100 | 0 | 0 |  | 0 | 100 | 0 | 0 |  | 90.2 | 0 | 0 | 9.8 |  |  |
| PHF | . 000 | . 375 | . 000 | . 000 | . 375 | . 000 | . 813 | . 000 | . 000 | 813 | . 925 | . 000 | . 000 | . 333 | . 932 | . 908 |



## APPENDIX B

Level of Service Definitions

## TECHNICAL MEMORANDUM

## Subject: Level of Service Definitions

## Introduction

The purpose of this memorandum is to define the level of service (LOS) metric that commonly used as a measure of effectiveness (MOE) for traffic operations.

All capacity analyses are based on the procedures specified by the Transportation Research Board's (TRB) Highway Capacity Manual (HCM), which is currently on its sixth edition. Level of service ranges from A to F. A brief description of each level of service for signalized and unsignalized intersections is provided below.

## Signalized Intersections

Level of service is based upon the traffic volume present in each lane on the roadway, the capacity of each lane at the intersection and the delay associated with each directional movement. The levels of service for signalized intersections are defined below:

- Level of Service A describes operations with very low average delay per vehicle, i.e., less than 10.0 seconds. This occurs when progression is extremely favorable, and most vehicles arrive during the green phase. Most vehicles do not stop. Short signal cycle lengths may also contribute to low delay.
- Level of Service B describes operations with average delay in the range of 10.1 to 20.0 seconds per vehicle. This generally occurs with good progression and/or short cycle lengths. More vehicles stop than for LOS A, causing higher levels of average delay.
- Level of Service C describes operations with delay in the range of 20.1 to 35.0 seconds per vehicle. These higher delays may result from fair progression and/or longer cycle lengths. Individual cycle failures may begin to appear at this level. The number of vehicles stopping is significant at this level although many still pass through the intersection without stopping. This is generally considered the lower end of the range of the acceptable level of service in rural areas.
- Level of Service D describes operations with delay in the range of 35.1 to 55.0 seconds per vehicle. At LOS D, the influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavorable progression, long cycle lengths, and/or high traffic volumes as compared to the roadway capacity. Many vehicles are required to stop and the number of vehicles that do not have to stop declines. Individual signal cycle failures, where all waiting vehicles do not clear the intersection during a single green time, are noticeable. This is generally considered the lower end of the range of the acceptable level of service in urban areas.
- Level of Service E describes operations with delay in the range of 55.1 to 80.0 seconds per vehicle. These higher delay values generally indicate poor progression, long cycle lengths, and high traffic volumes. Individual cycle failures are frequent occurrences. LOS E has been set as the limit of acceptable conditions.
- Level of Service F describes operations with average delay in excess of 80.0 seconds per vehicle. This is considered to be unacceptable to most drivers. This condition often occurs with over-saturation, i.e., when traffic arrives at a flow rate that exceeds the capacity of the intersection. It may also occur at high volumes with many individual cycle failures. Poor progression and long cycle lengths may also contribute to such delays.


## Unsignalized Intersections

At an unsignalized intersection, the major street through traffic and right-turns are assumed to operate unimpeded and therefore receive no level of service rating. The level of service for the minor street and the major street left-turn traffic is dependent on the volume and capacity of the available lanes, and, the number and frequency of acceptable gaps in the major street traffic to make a conflicting turn. The level of service grade is provided for each conflicting movement at an unsignalized intersection and is based on the total average delay experienced by each vehicle. The delay includes the time it takes a vehicle to move from the back of a queue through the intersection.

The unsignalized intersection level of service analysis does not account for variations in driver behavior or the effects of nearby traffic signals. Therefore, the results from this analysis usually indicate worse levels of service than may be experienced in the field. The unsignalized intersection level of service descriptions are provided below:

- Level of Service A describes operations where there is very little to no conflicting traffic for a minor side street movement, i.e., an average total delay of less than 10.0 seconds per vehicle.
- Level of Service B describes operations with average total delay in the range of 10.1 to 15.0 seconds per vehicle.
- Level of Service C describes operations with average total delay in the range of 15.1 to 25.0 second per vehicle.
- Level of Service D describes operations with average total delay in the range of 25.1 to 35.0 seconds per vehicle.
- Level of Service E describes operations with average total delay in the range of 35.1 to 50.0 seconds per vehicle.
- Level of Service $\mathbf{F}$ describes operations with average total delay of 50 seconds per vehicle. LOS $F$ exists when there are insufficient gaps of suitable size to allow a side street demand to cross safely through or enter a major street traffic stream. This level of service is generally evident from extremely long total delays experienced by side street traffic and by queuing on the minor approaches. It is important to note that LOS F may not always result in long queues but may result in adjustments to normal driver behavior.


## APPENDIX C

Intersection Capacity Analysis Results - Existing Conditions (2023)

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 1.8 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBU | EBL | EBT | EBR | WBU | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  |  | * $\uparrow$ |  |  | * | 中4 | 7 |  | * |  | ${ }^{7}$ |  | 「 |
| Traffic Vol, veh/h | 1 | 37 | 597 | 4 | 3 | 5 | 604 | 21 | 3 | 0 | 2 | 32 | 0 | 55 |
| Future Vol, veh/h | 1 | 37 | 597 | 4 | 3 | 5 | 604 | 21 | 3 | 0 | 2 | 32 | 0 | 55 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control F | Free | Free | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | - | None | - | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | 125 | - | 0 | - | - | - | 75 | - | 0 |
| Veh in Median Storage, \# |  | - | 0 | - | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | - | 0 | - | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 39 | 628 | 4 | 3 | 5 | 636 | 22 | 3 | 0 | 2 | 34 | 0 | 58 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 8.4 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Mr |  |  | - | F |  |
| Traffic Vol, veh/h | 0 | 77 | 15 | 0 | 1 | 0 |
| Future Vol, veh/h | 0 | 77 | 15 | 0 | 1 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 78 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 99 | 19 | 0 | 1 | 0 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 3 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBU | EBL | EBT | EBR | WBU | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  |  | * $\uparrow$ |  |  | \$ | 44 | 「 |  | $\uparrow$ |  | ${ }^{7}$ |  | 7 |
| Traffic Vol, veh/h | 1 | 61 | 770 | 1 | 1 | 3 | 640 | 41 | 3 | 0 | 20 | 33 | 0 | 50 |
| Future Vol, veh/h | 1 | 61 | 770 | 1 | 1 | 3 | 640 | 41 | 3 | 0 | 20 | 33 | 0 | 50 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control F | Free | Free | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | - | None | - | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | 125 | - | 0 | - | - | - | 75 | - | 0 |
| Veh in Median Storage, \# | \# | - | 0 | - | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | - | 0 | - | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 88 | 88 | 88 | 88 | 88 | 88 | 88 | 88 | 88 | 88 | 88 | 88 | 88 | 88 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 69 | 875 | 1 | 1 | 3 | 727 | 47 | 3 | 0 | 23 | 38 | 0 | 57 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 7.5 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Mr |  |  | $\uparrow$ | F |  |
| Traffic Vol, veh/h | 0 | 37 | 65 | 0 | 3 | 0 |
| Future Vol, veh/h | 0 | 37 | 65 | 0 | 3 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 91 | 91 | 91 | 91 | 91 | 91 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 41 | 71 | 0 | 3 | 0 |



## APPENDIX D

Intersection Capacity Analysis Results - No-Build Conditions (2027)

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh | 3.1 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBU | EBL | EBT | EBR | WBU | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  |  | ＊$\uparrow$ |  |  | ＊ | 中4 | F゙ |  | $\uparrow$ |  | ${ }^{7}$ |  | 「゙ |
| Traffic Vol，veh／h | 1 | 55 | 621 | 4 | 3 | 5 | 629 | 28 | 3 | 0 | 2 | 50 | 0 | 107 |
| Future Vol，veh／h | 1 | 55 | 621 | 4 | 3 | 5 | 629 | 28 | 3 | 0 | 2 | 50 | 0 | 107 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control F | Free | Free | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | － | None | － | － | － | None | － | － | None | － | － | None |
| Storage Length | － | － | － | － | － | 125 | － | 0 | － | － | － | 75 | － | 0 |
| Veh in Median Storage，\＃ | \＃ | － | 0 | － | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Grade，\％ | － | － | 0 | － | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 58 | 654 | 4 | 3 | 5 | 662 | 29 | 3 | 0 | 2 | 53 | 0 | 113 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 8.5 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | r |  |  | $-\uparrow$ | $\uparrow$ |  |
| Traffic Vol, veh/h | 0 | 147 | 38 | 0 | 1 | 0 |
| Future Vol, veh/h | 0 | 147 | 38 | 0 | 1 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, $\#$ | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 160 | 41 | 0 | 1 | 0 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 5.6 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBU | EBL | EBT | EBR | WBU | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  |  | * $\uparrow$ |  |  | \# | 中4 | F゙ |  | $\uparrow$ |  | ${ }^{7}$ |  | 7 |
| Traffic Vol, veh/h | 1 | 111 | 801 | 1 | 1 | 3 | 666 | 59 | 3 | 0 | 21 | 45 | 0 | 84 |
| Future Vol, veh/h | 1 | 111 | 801 | 1 | 1 | 3 | 666 | 59 | 3 | 0 | 21 | 45 | 0 | 84 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control F | Free | Free | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | - | None | - | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | 125 | - | 0 | - | - | - | 75 | - | 0 |
| Veh in Median Storage, \# | \# | - | 0 | - | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | - | 0 | - | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 121 | 871 | 1 | 1 | 3 | 724 | 64 | 3 | 0 | 23 | 49 | 0 | 91 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 7.8 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Mr |  |  | - | F |  |
| Traffic Vol, veh/h | 0 | 82 | 132 | 0 | 3 | 0 |
| Future Vol, veh/h | 0 | 82 | 132 | 0 | 3 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 89 | 143 | 0 | 3 | 0 |



## APPENDIX E

Intersection Capacity Analysis Results - Build Conditions (2027)

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 4.5 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBU | EBL | EBT | EBR | WBU | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  |  | * $\uparrow$ |  |  | \$ | 44 | F' |  | \& |  | ${ }^{7}$ |  | 「 |
| Traffic Vol, veh/h | 1 | 71 | 621 | 4 | 3 | 5 | 629 | 33 | 3 | 0 | 2 | 65 | 0 | 154 |
| Future Vol, veh/h | 1 | 71 | 621 | 4 | 3 | 5 | 629 | 33 | 3 | 0 | 2 | 65 | 0 | 154 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control F | Free | Free | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | - | None | - | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | 125 | - | 0 | - | - | - | 75 | - | 0 |
| Veh in Median Storage, \# |  | - | 0 | - | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | - | 0 | - | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 75 | 654 | 4 | 3 | 5 | 662 | 35 | 3 | 0 | 2 | 68 | 0 | 162 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.8 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | MF |  | 1 |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 47 | 0 | 43 | 16 | 0 | 163 |
| Future Vol, veh/h | 47 | 0 | 43 | 16 | 0 | 163 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, $\#$ | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 51 | 0 | 47 | 17 | 0 | 177 |


| Major/Minor M | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 233 | 56 | 0 | 0 | 64 | 0 |
| Stage 1 | 56 | - | - | - | - | - |
| Stage 2 | 177 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 |  | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 755 | 1011 | - | - | 1538 | - |
| Stage 1 | 967 | - | - | - | - | - |
| Stage 2 | 854 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 755 | 1011 | - | - | 1538 | - |
| Mov Cap-2 Maneuver | 755 | - | - | - | - | - |
| Stage 1 | 967 | - | - | - | - | - |
| Stage 2 | 854 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 10.1 |  | 0 |  | 0 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 755 | 1538 | - |
| HCM Lane V/C Ratio |  | - | - | 0.068 | - | - |
| HCM Control Delay (s) |  | - | - | 10.1 | 0 | - |
| HCM Lane LOS |  | - | - | B | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0.2 | 0 | - |


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 8.5 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | \$ |  |  | \$ |  |  | \& |  |
| Traffic Vol, veh/h | 0 | 0 | 147 | 15 | 0 | 0 | 38 | 0 | 5 | 0 | 1 | 0 |
| Future Vol, veh/h | 0 | 0 | 147 | 15 | 0 | 0 | 38 | 0 | 5 | 0 | 1 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 0 | 160 | 16 | 0 | 0 | 41 | 0 | 5 | 0 | 1 | 0 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh | 11.4 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBU | EBL | EBT | EBR | WBU | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  |  | ＊${ }^{\text {F }}$ |  |  | ＊ | 中4 | 「 |  | \＆ |  | ${ }^{7}$ |  | 「7 |
| Traffic Vol，veh／h | 1 | 155 | 801 | 1 | 1 | 3 | 666 | 73 | 3 | 0 | 21 | 55 | 0 | 115 |
| Future Vol，veh／h | 1 | 155 | 801 | 1 | 1 | 3 | 666 | 73 | 3 | 0 | 21 | 55 | 0 | 115 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control F | Free | Free | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | － | － | － | None | － | － | － | None | － | － | None | － | － | None |
| Storage Length | － | － | － | － | － | 125 | － | 0 | － | － | － | 75 | － | 0 |
| Veh in Median Storage，\＃ | \＃ | － | 0 | － | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Grade，\％ | － | － | 0 | － | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 168 | 871 | 1 | 1 | 3 | 724 | 79 | 3 | 0 | 23 | 60 | 0 | 125 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  |  | $\neq 1$ |
| Traffic Vol, veh/h | 31 | 0 | 146 | 44 | 0 | 95 |
| Future Vol, veh/h | 31 | 0 | 146 | 44 | 0 | 95 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, $\#$ | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 34 | 0 | 159 | 48 | 0 | 103 |





## APPENDIX F

Turn Lane Warrant Assessment

# Right Turn Lane Warrant Assessment <br> Two-Lane Highways 

Based on NCHRP Report 279 / VDOT RDM Appendix F
"Intersection Channelization Guide"

## Background:

Warrants for right-turn storage lanes on two-lane highways at unsignalized intersections are based on Figure 3-26 in Appendix F of the Virginia Department of Transportation's (VDOT) Road Design Manual (RDM). This figure provides a graphical representation for determining the necessity of a right turn lane and / or taper by comparing the total volumes of a given approach with their respective right turn volumes.

## Project Information:



| Study Scenario | Approach <br> Volume | Right Turn <br> Volume | Minimum Right <br> Turn Taper <br> Threshold | Minimum Right <br> Turn Full Lane <br> Threshold | Treatment |
| :--- | :---: | :---: | :---: | :---: | :---: |



## Gorove Slade

Transportation Engineers and Planners

## APPENDIX G

VDOT Pre-Scope Form

# PRE-SCOPE OF WORK MEETING FORM <br> Information on the Project <br> Traffic Impact Analysis Base Assumptions 

The applicant is responsible for entering the relevant information and submitting the form to VDOT and the locality no less than three (3) business days prior to the meeting. If a form is not received by this deadline, the scope of work meeting may be postponed.

## Contact Information

| Consultant Name: | Mike Bailey, P.E. - Gorove Slade Associates, Inc. |
| :---: | :--- |
| Tele: | $804-310-6040$ |
| E-mail: | mb@goroveslade.com |
| Developer/Owner Name: | John Neel - Foresight Design Services |
| Tele: | 1260 Radford Street |
| E-mail: | Christiansburg, VA 24073 |

## Project Information

| Project Name: | Houchins Road <br> Townhomes | Locality/County: | Montgomery County |  |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Project Location: <br> (Attach regional and site <br> specific location map) | See Figure 1 |  |  |  |
| Submission Type | Comp Plan $\square$ | REZ/SUP $\quad \boxtimes$ | Site Plan $\square$ | Subd Plat $\square$ |
|  | $\square$ |  |  |  |
| Project Description: <br> (Including details on the land <br> use, acreage, phasing, access <br> location, etc. Attach additional <br> sheet if necessary) | The proposed development is planned to occupy two parcels of land, which can be identified on <br> Montgomery County's Parcel Viewer with the Tax Map IDs 080-A46 and 080-A44. The two <br> parcels total approximately 26 acres and are both zoned as M-1 (Industrial Manufacturing) by <br> Montgomery County. <br> The proposed development plan includes the construction of 171 townhomes. The proposed site <br> access includes two full-movement driveways on Houchins Road (Route 758). |  |  |  |

## Traffic Impact Analysis Assumptions

| Study Period | Existing Year: 2023 |  |  | Build-out Year: 202 |  | Design Year: 2027 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Study Area Boundaries (Attach map) | North: See Figure 1 |  |  |  | South: |  |
|  | West: |  |  |  | East: |  |
| External Factors That Could Affect Project (Planned road improvements, other nearby developments) | TBD |  |  |  |  |  |
| Consistency With Comprehensive Plan (Land use, transportation plan) | The Montgomery County Comprehensive Plan lists this area as an urban expansion area. |  |  |  |  |  |
| Available Traffic Data (Historical, forecasts) | VDOT Historical AADT Data (2016-2021) |  |  |  |  |  |
| Trip Distribution <br> (Please refer to attached Figure 2 in Supplement) | Road Name: Roanoke Street (to/from the East) - 25\% |  |  |  | Road Name: |  |
|  | Road Name: Roanoke Street (to/from the West) - 75\% |  |  |  | Road Name: |  |
| Annual Vehicle Trip Growth Rate: | 1.0\% / yr |  |  | keriod for Study k all that apply) | $\boxtimes \mathrm{AM} \quad \boxtimes \mathrm{PM} \quad \square \mathrm{SAT}$ |  |
|  |  |  |  | k Hour of the Adj. used in study) |  |  |
| Study Intersections and/or Road Segments (Attach additional sheets as necessary) <br> (Please refer to attached Figure 1.) | Roanoke Street (U.S. 11/U.S. 460 <br> 1. BUS) at Houchins Road / Bristol <br> Drive |  |  |  | 7. |  |
|  | 2. | Houc Drive | oad | Proposed Site | 8. |  |
|  | 3. | Houc Propo | $\begin{aligned} & \text { oad } \\ & \text { te D } \end{aligned}$ | Crosscreek Drive / veway | 9. |  |
|  | 4. |  |  |  | 10. |  |
|  | 5. |  |  |  | 11. |  |
|  | 6. |  |  |  | 12. |  |
| Trip Adjustment Factors | Internal allowance Reduction:$\square$ Yes $\boxtimes$ No |  |  |  | Pass-by allowance Reduction:$\square$ Yes $\boxtimes$ No |  |
| Software Methodology | $\boxtimes$ Synchro $\square$ HCS |  |  | .2000/+) $\square$ SIDRA |  | $\square$ CORSIM $\square$ Other |


| Traffic Signal Proposed or Affected <br> (Analysis software to be used, progression speed, cycle length) | Existing traffic signals that could be affected: None Analysis Software: Synchro version 11 <br> Results: HCM 6 Methodology (See Note 6) Queue Lengths to be Reported: 95th Percentile |
| :---: | :---: |
| Improvement(s) <br> Assumed or to be <br> Considered | TBD |
| Background Traffic Studies Considered | Walnut Creek Residential - 22 single-family detached, 145 townhomes |
| Plan Submission | $\boxtimes$ Master Development Plan (MDP) $\square$ Generalized Development Plan (GDP) <br> $\square$ Preliminary/Sketch Plan $\square$ Other Plan type (Final Site, Subd. Plan) |
| Additional Issues to be Addressed | $\boxtimes$ Queuing analysis $\square$ Actuation/Coordination <br> $\square$ Weaving analysis  <br> $\square$ Merge analysis $\square$ Bike/Ped Accommodations $\boxtimes$ Intersection(s) <br> $\square$ TDM Measures $\square$ Other ( |

## NOTES on ASSUMPTIONS:

1. Traffic signal timings will be obtained from VDOT, if necessary.
2. The scenarios to be included in the study are Existing Conditions (2023), No-Build (2025), and Build (2025). The study will analyze the weekday AM and PM peak hours.
3. 2023 existing "baseline" condition counts will be collected at the study intersections.
4. Existing peak hour factors will be based on the traffic counts and utilized on a by-intersection basis. Peak hour factors by intersection in the range of 0.85 to 1.00 will be used for existing scenario. Peak hour factors of 0.92 will be used for all future scenarios if the existing peak hour factor by intersection is less than 0.92 .
5. Heavy vehicle percentages (HV\%) will be based on count data.
6. HCM 6 methodology will be utilized where applicable; HCM 2000 methodology will be utilized if HCM 6 methodology is not applicable.
7. Turn lane warrants will be assessed at the proposed site driveway(s).

Table 1: ITE Trip Generation (11 ${ }^{\text {th }}$ Edition)

| Land Use | $\begin{aligned} & \text { ITE } \\ & \text { Code } \end{aligned}$ | Size Units | AM Peak Hour |  |  | Weekday ----PM Peak Hour |  |  | Weekday Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | In | Out | Total | In | Out | Total |  |
| Single-Family Attached Housing | 215 | 171 d.u. | 21 | 62 | 83 | 58 | 41 | 99 | 1,254 |

Figure 1: Site Location and Study Intersections


Figure 2: Direction of Approach


Figure 3: Proposed Site Plan



[^0]:    Appendix E: Landscape and Buffer Plan

